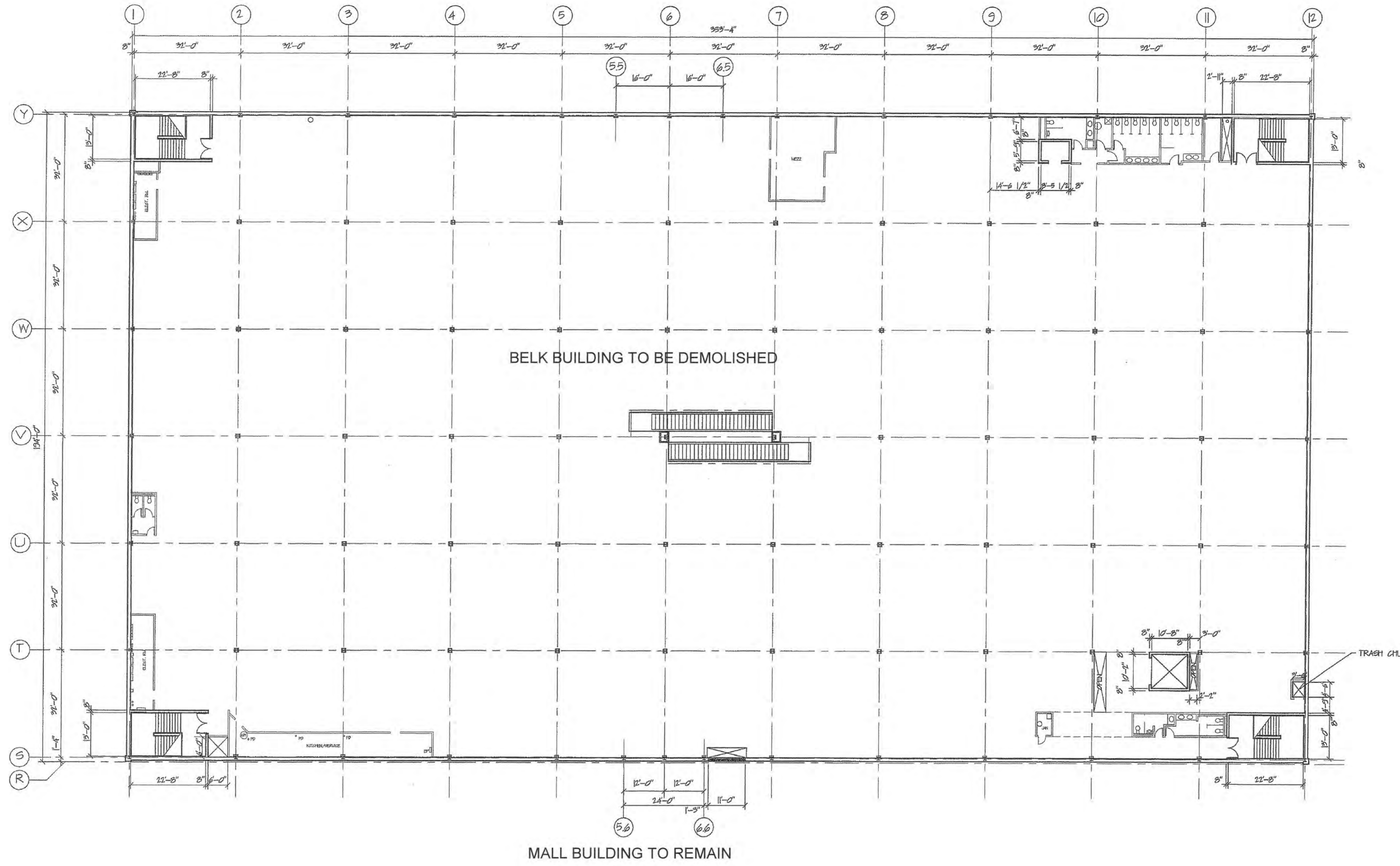




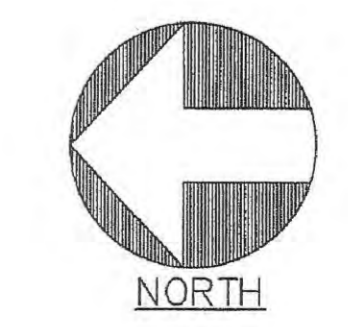
Seal



1 SECOND FLOOR PLAN  
A2 APPROXIMATE SCALE: 3/64" = 1'-0"

THIS DRAWING IS FOR REFERENCE ONLY. THE SCOPE OF WORK INCLUDES GENERAL DEMOLITION OF ALL INTERIOR AND EXTERIOR PORTIONS OF THE BUILDING. THE CONTRACTOR IS RESPONSIBLE FOR ALL GENERAL DEMOLITION INCLUDING REMOVAL OF THE SLAB. REFER TO SELECTIVE DEMOLITION ON DRAWING AD1.1. DP3 ARCHITECTS TAKES NO LIABILITY ON THE ACCURACY OF THIS REPRESENTATION OF THE ORIGINAL BELK BUILDING SHOWN. THE REMOVAL OF HAZARDOUS MATERIALS ASSOCIATED WITH THE DEMOLITION OF THE BELK BUILDING IS ASSOCIATED WITH PROJECT NUMBER H59-6124-CA-A.

THIS IMAGE IS ROTATED 180 DEGREES FROM ARCHITECTURAL DRAWINGS.



REV.	DATE	BY	DESCRIPTION
OVERALL PLAN- 2ND FL			
GREENVILLE TEC			
BELKS STORE			
McALISTER CENTER			
GREENVILLE SOUTH CAROLINA			
JOB NO:	0301	DRAWING NO.	
SHEET:	01	OF:	
DRAWN BY:	TRW	A2	
CHECKED BY:	RNG		
DATE:	01/08/03		

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Project  
**Greenville**  
Technical College  
Greenville - Demolition of  
Bldg. 602

State Project # H59-6124-CA-B  
Project Number 15288  
Drawn By Author  
Checked By Checker  
Date 12 Dec 2016

Revisions

Drawing

Existing Belk Bldg.  
Second Floor

RA 2

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## GENERAL NOTES

- 1.0 THIS PROJECT HAS BEEN DESIGNED FOR THE WEIGHTS AND MATERIALS INDICATED ON THE DRAWINGS AND FOR THE LIVE LOADS INDICATED IN THE DESIGN DATA.
- 1.1 COORDINATE THESE DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS.
- 1.2 THE CONTRACTOR SHALL REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS OR OPENINGS, ETC. NOT HEREIN INDICATED.
- 1.3 THE CONTRACTOR SHALL COORDINATE INSTALLATION OF ELEVATORS WITH EQUIPMENT MANUFACTURER. THIS INCLUDES ANY EMBEDDED ITEMS FOR GUIDE RAIL SUPPORTS, EDGE OF SLAB DIMENSIONS FOR CLEAR HOISTWAY, HOIST BEAMS, AND OTHER ITEMS REQUIRED FOR COMPLETE INSTALLATION OF ELEVATORS.
- 1.4 SLAB OPENINGS SMALLER THAN 10" AND NOT INDICATED ON PLAN SHALL BE CORE DRILLED IN FIELD U.N.O. SEE MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR LOCATIONS OF THESE OPENINGS.
- 1.5 WORK NOT INCLUDED ON THE DRAWINGS BUT IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES ELSEWHERE ON THE DRAWINGS SHALL BE REPEATED.
- 1.6 IN CASE OF CONFLICT BETWEEN THE NOTES, DETAILS AND SPECIFICATIONS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.
- 1.7 THE CONTRACTOR SHALL SUBMIT FOR REVIEW, DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A STRUCTURAL ENGINEER IN THE STATE OF SOUTH CAROLINA FOR THE METAL STAIRS AND RAILINGS. REVIEW SHALL BE FOR GENERAL CONFORMANCE TO LOCAL BUILDING CODES, DESIGN PARAMETERS LISTED IN THE GENERAL NOTES AND GEOMETRY DESIGNATED IN THE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING EMBEDS AND HARDWARE AS REQUIRED BY THE STAIR DESIGN.
- 1.8 SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY AND DRYWALL NON-LOAD BEARING PARTITIONS. PROVIDE COMPRESSIBLE FIRESAFING AT TOP OF WALL AS REQUIRED.
- 2.0 **FOUNDATION NOTES**
- 2.1 AN ASSUMED ALLOWABLE BEARING PRESSURE OF 2000 PSF IS BEING USED
- 2.2 SEE THE SPECIFICATION REQUIREMENTS FOR EXCAVATION AND PREPARATION OF THE FOUNDATION AND SLAB ON GRADE SUBGRADE INCLUDING COMPACTION PROCEDURES.
- 2.3 EXCAVATIONS FOR FOOTINGS SHALL HAVE THE SIDES AND BOTTOM TEMPORARILY LINED WITH 6 MIL POLYETHYLENE IF PLACEMENT OF CONCRETE DOES NOT OCCUR WITHIN 24 HOURS OF THE EXCAVATION OF THE FOOTING.
- 2.4 FOUNDATION CONDITIONS NOTED DURING CONSTRUCTION WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT SHALL BE REPORTED TO THE GENERAL CONTRACTOR BEFORE FURTHER CONSTRUCTION IS ATTEMPTED. SEE PROJECT SPECIFICATIONS. ALL BASEMENT WALLS UNLESS NOTED ARE DESIGNED AS LATERALLY SUPPORTED AT THE TOP. THEREFORE, THE 1ST FLOOR FRAME AND SLAB SHALL BE INSTALLED BEFORE BACKFILLING WALLS. ALSO, THE BASEMENT SLAB ON GRADE SHALL BE INSTALLED BEFORE BACKFILL AT PERIMETER BUILDING BASEMENT WALLS.
- 2.5 NO FOOTINGS OR SLABS SHALL BE POURED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER, FROST, ICE OR LOOSE MATERIAL.
- 2.6 SEE PLUMBING, ELECTRICAL & CIVIL DRAWINGS FOR REQUIRED UNDERSLAB UTILITIES.
- 2.7 SEE SPECIFICATIONS FOR ALL WATERPROOFING DETAILS AND MATERIALS AS REQUIRED.
- 2.8 IF UNDERMINING OF FOOTING OCCURS, FILL VOIDS WITH 2500 PSI CONCRETE. DO NOT ATTEMPT TO REPLACE AND RECOMPACT SOIL.
- 3.0 **CONCRETE**
- 3.1 CONCRETE SHALL HAVE THE UNIT WEIGHT AND THE MINIMUM COMPRESSIVE STRENGTHS ( $f_c$ ) AT 28 DAYS AS SHOWN ON THE CONCRETE MATERIALS SCHEDULE. (DWG S002) SEE SPECIFICATIONS FOR FURTHER INFORMATION.
- 3.2 ENTRAIN AIR TO PRODUCE TOTAL AIR CONTENT ACCORDING TO THE SPECIFICATIONS. FOR CONCRETE EXPOSED TO FREEZING TEMPERATURES (EXTERIOR FOOTINGS, SLAB TURNDOWNS, EXTERIOR SLABS AND SLABS-ON-GRADE, EXTERIOR RETAINING WALLS, AND EXTERIOR GRADE BEAMS.)
- 3.3 GROUT FOR BASE PLATES SHALL BE NON-SHRINKABLE GROUT AND SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5000 PSI, U.N.O.
- 3.4 NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE.
- 3.5 MIXING, TRANSPORTING AND PLACING OF CONCRETE SHALL CONFORM TO ACI-301.
- 3.6 ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI-318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", AND CONTRACT SPECIFICATIONS. WHEN THERE IS A CONFLICT BETWEEN ACI AND SPECIFICATIONS, THE MORE STRINGENT SHALL GOVERN.
- 3.7 CHAMFER ALL EXPOSED EXTERNAL CORNERS OF CONCRETE WITH 3/4" X 45 DEGREE CHAMFER U.N.O.
- 3.8 CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615, GRADE 60. REINFORCING BARS SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT, UNLESS INDICATED ON THE CONTRACT DOCUMENTS. ALL LAP SPLICES SHALL BE CLASS "B" U.N.O.
- 3.9 HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED WITH A CLASS B TENSION SPLICE AT CORNERS AND INTERSECTIONS. TOP BAR CRITERIA SHALL APPLY IF 12" OR MORE OF FRESH CONCRETE IS PLACED BELOW BAR.
- 3.10 SLABS-ON-GRADE SHALL HAVE CONSTRUCTION JOINTS OR CRACK CONTROL JOINTS AS SHOWN ON THE DRAWINGS. CONSTRUCTION JOINTS CAN BE USED AT CONTROL JOINT LOCATIONS AT CONTRACTORS OPTION AND VICE VERSA. SEE SLAB PLANS & JOINT DETAILS FOR ADDITIONAL INFORMATION.
- 3.11 SEE SPECIFICATIONS FOR ALL WATERPROOFING/DAMP-PROOFING DETAILS.
- 3.12 ALL WELDED WIRE FABRIC SHALL CONFORM TO THE STANDARDS OF ASTM A-185. SUPPLY IN FLAT SHEETS.
- 3.13 ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED, AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", ACI 318, AND THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315, LATEST EDITION.

- 3.14 SHOP DRAWINGS SHOWING REINFORCING DETAILS, INCLUDING STEEL SIZES, SPACING AND PLACEMENT SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION.
- 3.15 ALL WELDED WIRE FABRIC SHALL BE LAPPED TWO (2) FULL MESH PANELS AND TIED SECURELY.
- 3.16 ALL DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, U.N.O. ON DRAWINGS.
- 3.17 ADDITIONAL BARS SHALL BE PROVIDED AROUND ALL FLOOR AND WALL OPENINGS AS SHOWN ON THE DRAWINGS.
- 3.18 SEE ARCHITECTURAL DRAWINGS FOR TYPE AND LOCATION OF ALL FLOOR FINISHES.
- 3.19 THE CONTRACTOR SHALL COORDINATE ADDITIONAL WALL/SLAB OPENINGS NOT SHOWN ON STRUCTURAL DRAWINGS. SEE MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS.
- 3.20 U.N.O., ALL CURBS SHALL BE REINFORCED WITH AT LEAST 1 #4 CONTINUOUS AND #4 AT 12" O.C. DOWELS TO STRUCTURE BELOW.
- 3.21 THE SUB-CONTRACTOR SHALL VERIFY ALL OPENINGS, PAD SIZES, AND ANCHOR BOLTS WITH EQUIPMENT SELECTED.
- 3.22 FOR ALL WALLS & PIERS, PROVIDE DOWELS INTO FOOTING AT EACH VERT. REINF. BAR, U.N.O. DOWEL SIZE SHALL BE SAME AS VERT. REINF.
- 3.23 ALL DEFORMED BAR ANCHORS SHALL BE TRS NELSON DIVISION OR EQUAL 1/2" DIA. U.N.O. CONFORMING TO ASTM A-496 WITH A MINIMUM TENSILE STRENGTH OF 80,000 PSI. ANCHOR DIMENSIONS SHALL BE IN ACCORDANCE WITH ASTM D-19. INSTALL ANCHORS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS BY AUTOMATIC END WELDING AS INDICATED ON THE DRAWINGS. NO UNAUTHORIZED OR FIELD WELDING SHALL BE MADE WITHOUT AUTHORIZATION FROM THE MANUFACTURER.
- 3.24 ALL REINFORCING INDICATED TO BE WELDED SHALL BE IN ACCORDANCE WITH ASTM A706, "LOW ALLOY STEEL DEFORMED BARS FOR CONCRETE REINFORCEMENT". ANY INSTALLATIONS USING MANUFACTURER'S EQUIPMENT SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
- 3.25 PROVIDED CONCRETE POUR STOPS OR FORM AS REQUIRED FOR INSTALLATION OF ALL CONCRETE WORK.
- 3.26 PROVIDE ADDITIONAL 2-#4 x 4'-0" REINFORCING BARS IN SLAB-ON-GRADE AT ALL RE-ENTRANT CORNERS. PLACE BARS AT MID-DEPTH OF SLAB WITH A CLEARANCE OF 2" FROM CORNER U.N.O.
- 4.0 **CONCRETE MASONRY**
- 4.1 MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF THESE CONTRACT DOCUMENTS AND THE PROJECT SPECIFICATIONS.
- 4.2 THE SPECIFIED ULTIMATE COMPRESSIVE STRENGTH OF CONCRETE MASONRY ( $f_m$ ) ON THE NET AREA IS A MINIMUM OF 1500 PSI.
- 4.3 PROVIDE TWO #5 BARS CONTINUOUS IN ALL BOND BEAMS, UNLESS OTHERWISE INDICATED IN THE DRAWINGS. REINFORCEMENT PLACED IN BOND BEAMS SHALL BE CONTINUOUS WITH STANDARD ACI HOOKS AT EACH END. PROVIDE STANDARD BAR SPLICES AS SPECIFIED. MAXIMUM BOND BEAM SPACING SHALL BE 8'-0" O.C. U.N.O.
- 4.4 PROVIDE VERTICAL REINFORCEMENT IN ALL WALLS AS SHOWN. PROVIDE VERTICAL BARS AT EACH END OF ALL WALLS AS SHOWN. PROVIDE VERTICAL BARS ON EACH SIDE OF OPENINGS IN ALL WALLS AS SHOWN. PROVIDE STANDARD BAR SPLICES AS SPECIFIED. ALL VERTICAL REINFORCEMENT EXTENDS FULL HEIGHT OF WALL. SEE TYPICAL MASONRY DETAIL DWG. S501 AND OTHER SECTIONS AND DETAILS AS INDICATED ON DRAWINGS.
- 4.5 BLOCK CELLS THAT REQUIRE VERTICAL REINFORCING BARS AS INDICATED ON THE CONTRACT DRAWINGS AND/OR SPECS SHALL BE PLACED IN CENTERS OF BLOCK CELLS U.N.O.
- 4.6 PROVIDE JOINT REINFORCEMENT AS INDICATED IN THE SPECIFICATIONS AND ON THE ARCHITECTURAL DRAWINGS.
- 4.7 PROVIDE CONTROL JOINTS AT MAJOR CHANGES IN WALL HEIGHT, CHANGES IN WALL THICKNESS, AT FLOOR CONTROL JOINTS, AT WALL OPENINGS, AND NEAR RETURN ANGLES OF L, T, AND U SHAPED STRUCTURES. CONTROL JOINT SPACING SHALL NOT EXCEED THE DISTANCES INDICATED ON THE ARCHITECTURAL DRAWINGS.
- 4.8 GROUT FOR MASONRY SHALL BE NORMAL WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. GROUT SHALL CONFORM TO ASTM C476. GROUT LIFTS SHALL NOT EXCEED 4'-0".
- 4.9 FILL ALL BOND AND LINTEL BEAMS AND CELLS AT VERTICAL REINFORCEMENT WITH GROUT.
- 4.10 USE MORTAR TYPE AS INDICATED IN THE SPECIFICATIONS.
- 4.11 ALL CELLS, OPEN CAVITIES, AND AIR SPACES BELOW GRADE SHALL BE GROUTED.
- 4.12 BOND BEAMS AND REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS U.N.O. MAXIMUM CONTROL JOINT SPACING SHALL BE AS INDICATED ON THE ARCHITECTURAL DRAWINGS.
- 4.13 CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS. SEE ARCH., MECH., ELEC., AND PLUMBING DWGS. FOR SIZE AND LOCATION OF OPENINGS. SEE DRAWINGS FOR TYPICAL MASONRY SECTIONS AND DETAILS.
- 5.0 **STEEL DECK**
- 5.1 STEEL DECK SHALL BE ASTM A446 HAVING A MINIMUM YIELD STRENGTH OF 33,000 PSI AS PER THE STEEL DECK INSTITUTE DESIGN MANUAL.
- 5.2 STEEL DECK SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AND ERECTION LAYOUTS AND CONNECTED TO SUPPORTING MEMBERS AS INDICATED BELOW.
- 5.3 **ROOF DECK**
- 5.3.1 STEEL ROOF DECK SHALL BE 1 1/2", 22 GAGE GALVANIZED (UNPAINTED) WIDE RIB (S.D.I. TYPE WR 22) U.N.O.
- 5.3.2 ROOF DECK SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:
- | <u>22 GAGE</u>                        |                               |
|---------------------------------------|-------------------------------|
| MOMENT OF INERTIA, $I_p$              | 0.155 INFT <sup>4</sup> WIDTH |
| MOMENT OF INERTIA, $I_n$              | 0.183 INFT <sup>4</sup> WIDTH |
| SECTION MODULUS (TOP OF DECK), $S_n$  | 0.192 INFT <sup>3</sup> WIDTH |
| SECTION MODULUS (BOT. OF DECK), $S_p$ | 0.186 INFT <sup>3</sup> WIDTH |

- 5.3.3 IN ADDITION TO MEETING THE MINIMUM REQUIREMENTS ABOVE, THE DECK MANUFACTURER SHALL DESIGN THE ROOF DECK AND ATTACHMENTS TO STEEL FOR THE ROOF LOADS, INCLUDING DECK UPLIFT. ALL ROOF DECK SHALL HAVE A MINIMUM 3-SPAN CONDITION.
- 5.3.4 ROOF DECK SHALL BE WELED TO SUPPORTS WITH #12 TEK IN THE BOTTOM OF THE FLUTES USING A S.D.I. 387 PATTERN U.N.O. DECK SIDELAPS SHALL BE FASTENED USING #10 SCREWS WITH A MINIMUM 7-SIDE LAPS CONNECTIONS PER SPAN. ALL ENDLAPS SHALL BE A MINIMUM OF 2" AND SHALL OCCUR OVER SUPPORTS. MINIMUM DIAPHRAGM SHEAR STRENGTH  $Q = 400$  PLF U.N.O.
- 5.3.5 DO NOT SUSPEND PIPES, DUCTS, OR CEILING FROM ROOF DECK.
- 6.0 **STRUCTURAL STEEL**
- 6.1 STRUCTURAL STEEL ROLLED SHAPES AND PLATES SHALL CONFORM TO THE MATERIAL INFORMATION SCHEDULE ON SHEET S002. DIMENSIONS AND PROPERTIES SHALL BE IN ACCORDANCE TO ASTM A6.
- 6.2 ANCHOR BOLTS SHALL CONFORM TO ASTM A36, OR A307 U.N.O.
- 6.3 CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL BE 3/4" DIA A325-N, U.N.O. AND SHALL CONFORM TO ASTM A325; NUTS SHALL CONFORM TO ASTM A563; WASHERS SHALL CONFORM TO ASTM F436. CONNECTION BOLTS SHALL HAVE A HARDENED WASHER PLACED UNDER THE ELEMENT TO BE TIGHTENED.
- 6.4 DETAILING OF STRUCTURAL STEEL CONNECTIONS MUST BE CONSISTENT WITH RECOGNIZED, PUBLISHED METHODS SUCH AS IN THE AISC "MANUAL OF STEEL CONSTRUCTION", CURRENT EDITION, "ENGINEERING FOR STEEL CONSTRUCTION", OR "VOLUME II CONNECTIONS MANUAL OF STEEL CONSTRUCTION."
- 6.5 SECTION A7 OF AISC CURRENT EDITION IS AMENDED SUCH THAT THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF ALL CONNECTIONS.

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Seal



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Project

Greenville -  
 Demolition of  
 Bldg. 602

State Project # **H59-6124-CA-B**  
 Project Number 15288  
 Drawn By ERB  
 Checked By MMS  
 Date 12 Dec 2016

Revisions

Drawing

GENERAL NOTES

**S001**

**GENERAL NOTES (CONTINUED):**

- 6.6 SEE DWG. S401 FOR STANDARD FRAMING CONNECTIONS. STRUCTURAL STEEL CONNECTIONS SHALL BE DETAILED BY THE CONTRACTOR IN ACCORDANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION-ALLOWABLE STRESS DESIGN", CURRENT EDITION. CONNECTIONS SHALL BE DESIGNED TO DEVELOP A MINIMUM END REACTION OF 12.0 KIPS.
- 6.7 ALL CONNECTIONS FOR BEAMS WHICH SUPPORT A CONCRETE SLAB (COMPOSITE BEAMS) SHALL BE DESIGNED USING THE END REACTION SHOWN ON THE FRAMING PLANS OR NOTE 6.8 ON S002.
- 6.8 U.N.O. AS THUS; ~~###K~~ EITHER IN PLAN OR IN THE BEAM REACTION TABLES, CONNECTIONS SHALL BE DESIGNED AND DETAILED FOR THE END REACTION DETERMINED FROM PART 2 - "ALLOWABLE UNIFORM LOAD TABLES" FROM THE AISC MANUAL OF STEEL CONSTRUCTION - ASD CURRENT EDITION OR A MINIMUM OF 18 KIPS, WHICHEVER IS GREATER.
- 6.9 ALL MEMBERS AND CONNECTIONS ON THE CONTRACT DRAWINGS AND CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT SHOWN SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA DETAILED AND SUBMITTED FOR APPROVAL AND SHOWN ON THE SHOP DRAWINGS.
- 6.10 ALTERNATIVE CONNECTION DETAILS MAY BE SUBMITTED ON SHOP DRAWINGS BY THE CONTRACTOR ONLY IF ACCOMPANIED BY COMPLETE STRUCTURAL CALCULATIONS PREPARED AND SEALED BY AN ENGINEER, LICENSED IN THE STATE OF SOUTH CAROLINA AND SUBMITTED FOR APPROVAL.
- 6.11 CALCULATIONS FOR DETAILS MUST SHOW A RATIONAL ANALYSIS OF A COMPLETE LOAD PATH, INCLUDING LOCAL EFFECTS ON WEBS, FLANGES, ETC. OF THE CONNECTED MEMEBERS AND THE DEVICES (PLATES, SEATS, BRACKETS, BOLTS, WEBS, ETC) AFFECTING ALL CONNECTIONS. FAILURE TO SUBMIT SUCH CALCULATIONS FOR REVIEW CONCURRENT WITH SHOP DRAWING ERECTION PLANS AND DETAILS WILL BE CAUSE FOR RESUBMITTAL.
- 6.12 ALL SHEAR TAB CONNECTIONS SUBMITTED AS AN ALTERNATE FOR APPROVAL SHALL BE DESIGNED USING A FLEXIBLE SUPPORT CONDITION.
- 6.13 BEAM AND GIRDER CONNECTIONS SHALL BE DESIGNED SUCH THAT ALL ADDITIONAL STRESSES DUE TO CONNECTION ECCENTRICITY SHALL BE DEVELOPED BY THE CONNECTION AND NOT INCODUCE ANY ADDITIONAL STRESSES INTO SUPPORTING MEMEBERS.
- 6.14 STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN" (CURRENT EDITION), AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (CURRENT EDITION).
- 6.15 WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE AWS D1.1. ELECTRODES FOR SHOP AND FIELD WELDS SHALL BE CLASS E70XX. ALL WELDING SHALL BE DONE BY QUALIFIED, CERTIFIED WELDERS PER THE ABOVE STANDARD.
- 6.16 SHOP AND FIELD TESTING OF WELDS AND BOLTS SHALL BE AS OUTLINED IN THE SPECIFICATIONS.
- 6.17 ALL WELDS NOT INDICATED SHALL BE A MINIMUM OF 1/4" ALL AROUND U.N.O.
- 6.18 THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMEBERS FOR THE WORK OF OTHER TRADES WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
- 6.19 FOR FLOOR AND ROOF OPENINGS, THE FABRICATOR SHALL VERIFY OPENING LOCATIONS WITH EQUIPMENT SELECTED AND MAKE ANY NECESSARY MODIFICATIONS AT NO ADDITIONAL COST. THE CONTRACTOR SHALL COORDINATE MECHANICAL UNITS AND OPENINGS AND ARCHITECTURAL ITEMS REQUIRED FOR COMPLETE INSTALLATION OF WORK. IT IS THE RESPONSIBILITY OF FABRICATOR TO RECEIVE ALL NECESSARY INFORMATION PRIOR TO FABRICATION OF THE STEEL.
- 6.20 ALL STRUCTURAL STEEL WITH IS TO BE SPRAYED WITH FIREPROOFING SHALL NOT BE PRIMED OR PAINTED. STEEL WHICH IS NOT SPRAYED WITH FIREPROOFING SHALL BE PRIMED AND PAINTED PER SPECIFICATIONS. FOR STEEL BEAMS THAT ARE PRIMED, THE TOP FLANGE RECEIVING STEEL STUDS SHALL NOT BE PRIMED PAINTED.
- 6.21 ALL PLATES NOT INDICATED SHALL BE 5/16" MIN. THICKNESS. ALL ANGLES NOT INDICATED SHALL BE 3x3x5/16" MIN.
- 6.22 SEE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR ADDITIONAL OPENINGS NOT SHOWN. SEE DWG. S401 FOR TYPICAL FRAMING DETAILS.
- 6.23 FOR ARCHITECTURAL WELDS PROVIDE 3/16" FILLET OR STITCH WELD 2" LONG AT 12" O.C. U.N.O. MIN. WHERE WELDS ARE PERMANENTLY EXPOSED TO VIEW PROVIDE CONTINUOUS WELD IN ACCORDANCE WITH NOTE 6.18.
- 7.0 NOT USED

- 8.0 STEEL JOISTS
- 8.1 THE DETAILING, FABRICATION AND ERECTION OF STEEL JOISTS SHALL CONFORM TO THE LATEST STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE. JOISTS SHALL EXTEND TO WITHIN 5/16" OF CENTERLINE OF SUPPORTING BEAMS AT ABUTING JOISTS SPANS AND AT LEAST 2" PAST CENTERLINE OF SUPPORTING BEAMS ELSEWHERE AND IN ANY CASE JOIST ENDS SHALL BEAR A MINIMUM OF 2 1/2" ON STEEL SUPPORTS AND SHALL BE ANCHORED WITH A MINIMUM OF TWO 3/16"x1" FILLET WELDS (OR BOLTED CONNECTION) AND A MINIMUM OF 4" ON MASONRY OR CONCRETE SUPPORTS, BEARING ON A STEEL PLATE. BRIDGING SHALL BE AS SHOWN ON PLANS AS A MINIMUM AND WELDED TO INSIDE OF TOP AND BOTTOM CHORDS AND ANCHORED IN ACCORDANCE WITH SJI SPECIFICATIONS AT THEIR ENDS. EXTEND TOP AND BOTTOM CHORDS AS REQUIRED.
- 8.2 WHERE BOTTOM CHORD EXTENSIONS ARE REQUIRED, ALL DEAD LOADS SHALL BE IN PLACE PRIOR TO ATTACHMENT OF BOTTOM CHORD TO ADJACENT MEMBER.
- 8.3 FOR CONCENTRATED LOADS GREATER THAN 200 LBS ON JOIST, NOT FALLING DIRECTLY OVER A JOIST CHORD PANEL POINT, PROVIDE WEB REINFORCING ANGLES FROM THE SUPPORT POINT TO A BOTTOM CHORD PANEL POINT. USE L 2x2x3/16" OF EACH SIDE OF JOIST AND USE 3/16" FILLET WELD OR LEAST THICKNESS OF MATERIAL USED (WHICHEVER IS LESS), U.N.O. MAXIMUM POINT LOAD NOT TO EXCEED 500 LBS U.N.O.
- 8.4 WHERE JOISTS ARE ADJACENT TO WIDE FLANGE BEAMS, ALL LINES OF BRIDGING SHALL BE ANCHORED TO THE WIDE FLANGE BEAMS.
- 8.5 ALL ROOF JOISTS SHALL BE PROVIDED WITH SINGLE LINES OF BOTTOM CHORD BRIDGING FOR UPLIFT AT FIRST BOTTOM CHORD PANEL POINT AT EA END OF JOIST, FOR CLARITY, THIS BRIDGING IS NOT SHOWN ON THE ROOF FRAMING PLANS.
- 8.6 THE CAMBER FOR STEEL JOISTS SHALL BE AS SPECIFIED BY JOIST MANUFACTURER TO SUSTAIN THE LOADS AND CONDITIONS AS SPECIFIED.
- 8.7 STEEL JOISTS FOR ROOFS SHALL BE DESIGNED FOR A NET UPLIFT AS SPECIFIED. ADDITIONAL BRIDGING MAY BE ADDED IF REQUIRED BY STEEL JOIST MANUFACTURER.
- 8.8 OPENINGS FOR DUCTS, VENTS, MECHANICAL EQUIPMENT, ETC. ARE TO BE FRAMED PER DWG. S308 UNLESS SHOWN OTHERWISE. MECHANICAL CONTRACTOR TO PROVIDE EXACT SIZES AND LOCATIONS FOR OPENINGS TO BE COORDINATED BY THE JOIST FABRICATOR.
- 8.9 STEEL JOIST SIZES INDICATED ARE BASED UPON THE GRAVITY LOADS REQUIRED. SPECIAL JOISTS OR JOISTS OF LARGER SIZES SHALL BE PROVIDED AS REQUIRED BY THE STEEL JOIST MANUFACTURER TO ACCOMMODATE SITUATIONS AND LOADS INDICATED ON THE DRAWINGS, SUCH AS HANGER LOADS, UPLIFT LOADS, WIND LOADS, CHORD EXTENSION LOADS, ETC. ALL JOIST SIZES SHOWN ON DRAWINGS ARE GIVEN AS MINIMUM AND SHALL MEET THE MINIMUM UNIFORM LOAD CARRY CAPACITIES AS SPECIFIED BY THE STEEL JOIST INSTITUTE. STEEL JOISTS DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA.
- 8.10 JOISTS SHALL BE DESIGNED FOR LOAD REVERSAL AS REQUIRED.
- 8.11 JOISTS SHALL BE DESIGNED FOR CONCENTRATED LOADS APPLIED CONCURRENTLY WITH THE APPLICABLE LOAD CONDITIONS INCLUDING ROOF LIVE LOADS. BOTTOM CHORD PANEL POINTS SHALL BE DESIGNED FOR MAXIMUM HANGER LOADS OF 500 LBS APPLIED AT ANY PANEL POINT.
- 8.12 ALL STEEL JOISTS SHALL BE PRIMED UNLESS FIREPROOFED.
- 8.13 ALL BRIDGING SHOWN IS THE MINIMUM REQUIRED.
- 8.14 MINIMUM JOIST SEAT END BEARING BOUNDARY SHEAR "Q" = 400 LF U.N.O. ALL K-SERIES JOIST SEATS SHALL BE 2 1/2" MIN. U.N.O.
- 9.0 NOT USED
- 10.0 LIGHTGAUGE STEEL FRAMING NOTES
- 10.1 ALL MEMBER DESIGN, SIZES AND NOMENCLATURE, ETC. WHERE SHOWN ON DRAWINGS ARE BASED ON STEEL STUD MANUFACTURERS ASSOCIATION'S (SSMA) PRODUCT TECHNICAL INFORMATION UTILIZING AISI 1996 COLD FORMED STEEL DESIGN MANUAL. ANY VARIATION FROM SSMA INFORMATION REQUIRES AN APPROVAL WITH COMPLETE TECHNICAL DATA, INCLUDING PROFILES, DESIGN CALCULATIONS, ETC. STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE BUILDING IS TO BE CONSTRUCTED.
- 10.2 ALL STUDS, TRACKS AND JOISTS OF 18, 16, 14 OR 12 GAUGE TO BE CORROSION RESISTANT A653 CLASS 1 WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI.
- 10.3 ALL STUDS, TRACKS AND JOISTS OF 20 OR 22 GAUGE TO BE CORROSION RESISTANT A653 GRADE 33, WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI.
- 10.4 ALL BRIDGING AND CONNECTION PIECES TO BE CORROSION RESISTANT A653 GRADE 33, WITH A MINIMUM YIELD STRENGTH OF 33,000 PSI.
- 10.5 STUD AND JOISTS CONNECTIONS TO STUDS AND JOISTS THAT ARE TO BE WELDED SHALL BE BY AWS LIGHT GAUGE CERTIFIED WELDERS. MIN. WELD LENGTH SHALL BE 1". ALL WELDS SHALL BE WIRE BRUSHED AND COATED WITH A ZINC RICH PRIMER OR GALVANIZING REPAIR PRODUCT.
- 10.6 USE THREE (3) STUDS (MINIMUM U.N.O.) AT CORNERS OF ALL EXTERIOR WALLS AND USE 2 FULL HEIGHT STUDS EACH SIDE OF OPENINGS.
- 10.7 WHEN SCREWS ARE USED FOR CONNECTIONS, THE MINIMUM SPACING IS 3/4" AND 1/2" FOR CLEARANCE TO MEMBER EDGE.
- 10.8 TRACK, BRIDGING AND CONNECTION PIECES TO BE WELDED OR SCREWED USING SELF TAPPING S-12 SCREWS, MIN. NO. 8.

- 10.9 ATTACHMENT OF TRACK OR CONNECTION PIECES TO CONCRETE SHALL BE AS SHOWN ON DESIGN DRAWINGS.
- 10.10 LATERAL BRIDGING SHALL BE PROVIDED WHERE SHOWN, AS A MINIMUM. CONNECT BRIDGING TO METAL STUDS.
- 10.11 NO SPLICES ARE ALLOWED IN STUDS OR JOISTS. SPLICES IN TRACK OR CONTINUOUS CONNECTION PIECES SHALL SUPPLY THE FULL STRENGTH OF THE MEMBER STUDS.
- 10.13 ALL MEMBERS ARE TO BE INSTALLED PLUMB, LEVEL OR IN LINE WITH THE SLOPE OF THE STRUCTURE.
- 10.14 DURING CONSTRUCTION, TEMPORARY ERECTION BRACING, SHORING AND/OR SUPPORTS SHALL BE PROVIDED AS REQUIRED TO INSURE STRUCTURAL STABILITY UNTIL ALL STRUCTURAL COMPONENTS ARE PROPERLY INSTALLED, ALIGNED AND SECURED.
- 10.15 LIGHT GAGE STEEL MEMBERS SIZES SHOWN ON DWGS. ARE MINIMUM SIZES. GENERAL CONTRACTOR SHALL OBTAIN ENGINEER TO DESIGN ALL LIGHT GAGE STEEL FRAMING SHOWN ON STRUCTURAL AND ARCHITECTURAL DRAWINGS. SHOP DRAWINGS AND CALCULATIONS SEALED BY ENGINEER REGISTERED IN SOUTH CAROLINA SHALL BE SUBMITTED FOR APPROVAL FOR ALL FRAMING.
- 10.16 MAXIMUM DEFLECTION SHALL NOT EXCEED L/800 FOR BRICK VENEER AND L/360 OTHERWISE. (SEE SPECS FOR ADDITIONAL INFORMATION.)
- 11.0 METAL WALKWAY COVERS AND CANOPIES
- 11.1 DESIGN CALCULATIONS SHOWING ALL INPUT DATA AND REACTIONS SHALL BE SUBMITTED FOR REVIEW WITH THE METAL WALKWAY COVER/CANOPY SHOP DRAWINGS. ALL CALCULATIONS SHALL HAVE THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF SOUTH CAROLINA, ALL FRAME REACTIONS TO BE FURNISHED TO ENGINEER.
- 11.2 ALL METAL WALKWAY COVER/CANOPY COMPONENTS SHALL BE DESIGNED BASED ON THE SPECIFICATIONS AND ON THE FOLLOWING LOADS AND CRITERIA:
  - DEAD LOADS:
    - FRAME, ROOFING AND PURLINS - PER MANUFACTURER.
    - ADDITIONAL COLLATERAL LOADS: 4 PSF U.N.O.
  - LIVE LOADS:
    - 20 PSF WITH STANDARD BUILDING CODES TRIBUTARY AREA LIVE LOAD REDUCTION ALLOWED U.N.O.
  - WIND:
    - PER IBC 2015 AND AMENDMENTS LISTED ABOVE AND 155 MPH WIND VELOCITY.
  - SEISMIC:
    - PER IBC 2015 AND AMENDMENTS LISTED ABOVE WITH COEFFICIENTS AND CATEGORIES GIVEN THE DESIGN LOADS AND CRITERIA NOTES.

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 Bldg. 602

State Project # **H59-6124-CA-B**  
 Project Number 15288  
 Drawn By ERB  
 Checked By MMS  
 Date 12 Dec 2016

Revisions

Drawing

GENERAL NOTES

**S002**

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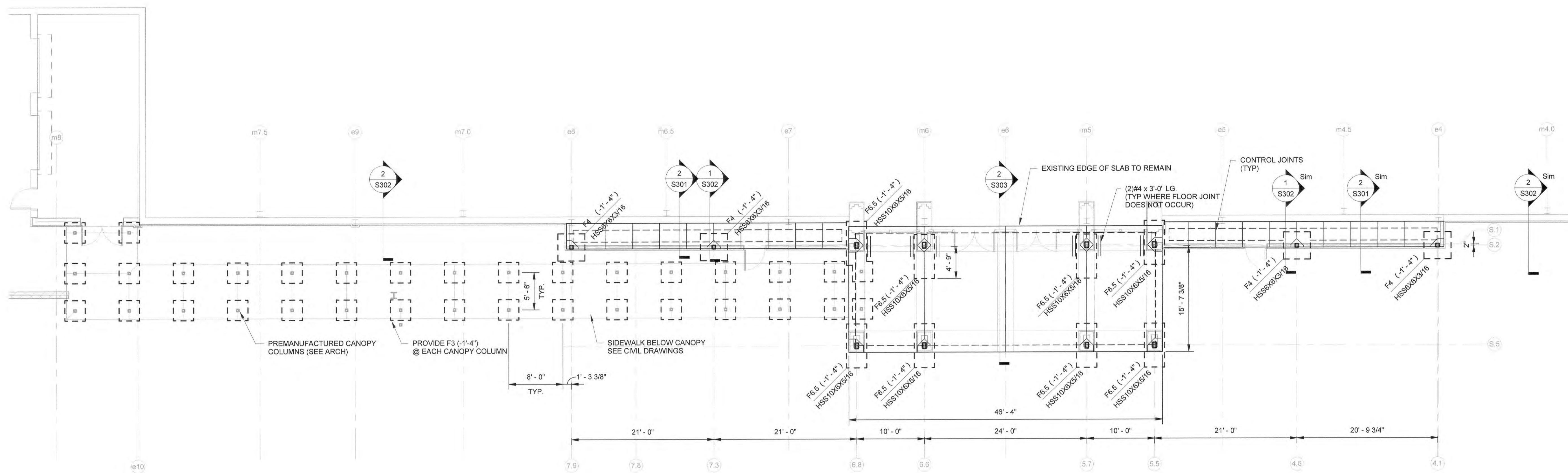
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Revisions



**1 FIRST FLOOR**  
 S101 1/8" = 1'-0"

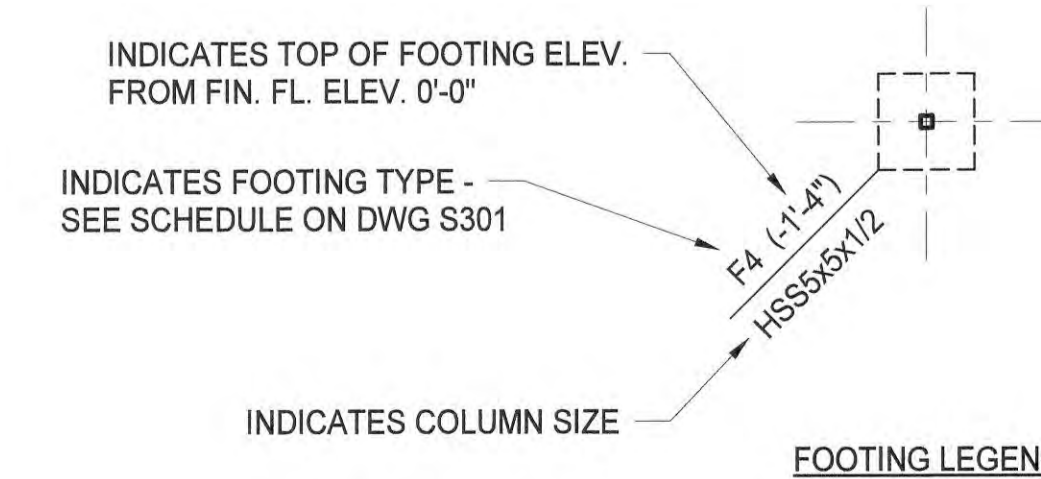
BUILDING FINISH FLOOR EL. 0'-0" (U.N.O.) + DATUM EL. xx'-x" (SEE CIVIL DWG)

SLAB-ON-GRADE = 4" CONCRETE SLAB REINF. W/ WWF 6x6xW1.4xW1.4 OVER 15 MIL VAPOR BARRIER AND 4" - #57 STONE BASE. (TYP, U.N.O.)

CJ - INDICATES CONSTRUCTION JOINT, SEE DETAIL 1/S301  
 CTJ - INDICATES CONTROL JOINT, SEE DETAIL 1/S301

**NOTES:**

- SEE DRAWING S001 - S003 FOR GENERAL NOTES AND BASIS OF DESIGN.
- SEE ARCH. DRAWINGS FOR ALL BUILDING/WALL PLAN DIMENSIONS NOT SHOWN ON STRUCTURAL DWGS.
- SEE ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS.
- ALL FOOTINGS SHALL BE CENTERED ON COLUMNS, U.N.O.
- CONTINUOUS WALL FOOTING REINFORCEMENT AND TURNED DOWN GRADE BEAMS, LONG-WAY BARS, SHALL BE CONTINUOUS THRU COLUMN FOOTINGS.
- WHERE CONTINUOUS FOOTINGS OF DIFFERENT SIZES MERGE TOGETHER, THE SMALLER FOOTING LONG-WAY BARS SHALL EXTEND, (TENSION LAP SPLICE), INTO THE LARGER FOOTING.
- COORDINATE ALL SLAB RECESS LOCATIONS, SIZE AND DEPTHS W/ ARCH. DWGS.
- SEE PLUMBING DRAWINGS FOR ADD'L INFORMATION.
- PLACE CJ AT INTERIOR/EXTERIOR SLAB TRANSITION LOCATIONS.



Drawing

FOUNDATION /  
 SLAB PLAN



**S101**

Seal



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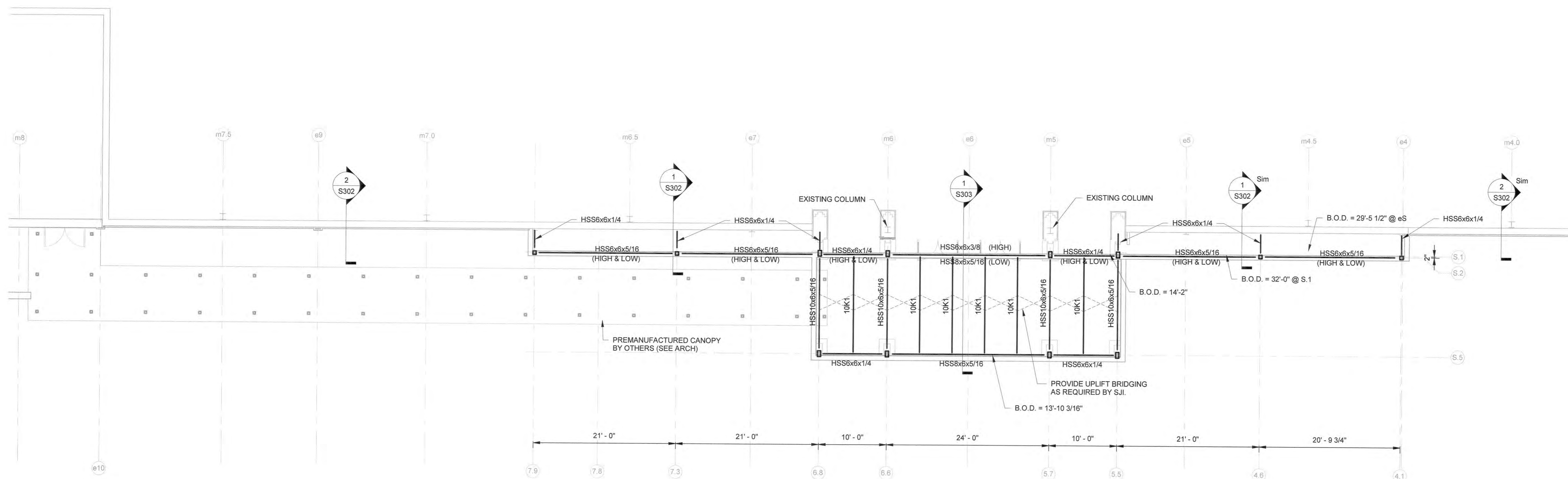
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**1** ROOF FRAMING PLAN  
 S201 1/8" = 1'-0"

T/STL ELEV. = SEE PLAN

ALL ROOF DECKING IS 1 1/2", 22 GAGE TYPE B GALV. METAL DECK (TYP, UNO).

NOTE: ALL JOIST SPACED AT 5'-0" O.C. MAX (TYP, U.N.O)

NOTES:  
 1. SEE S001 - S003 FOR GENERAL NOTES AND BASIS OF DESIGN  
 2. SEE ARCH. DWGS. FOR ALL BUILDING/WALL PLAN DIMENSIONS NOT SHOWN.

Drawing

ROOF FRAMING  
 PLAN



**S201**

Seal



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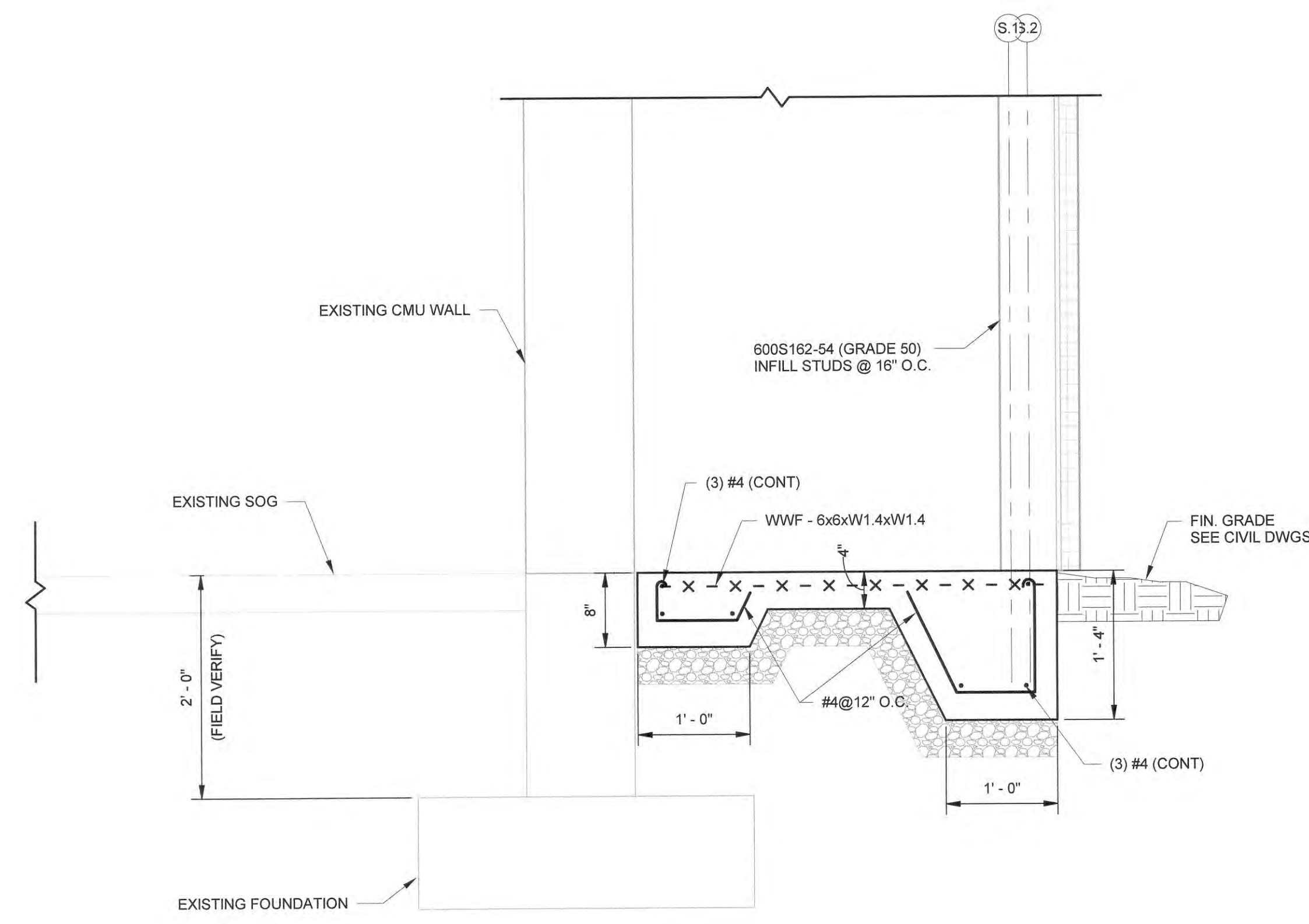
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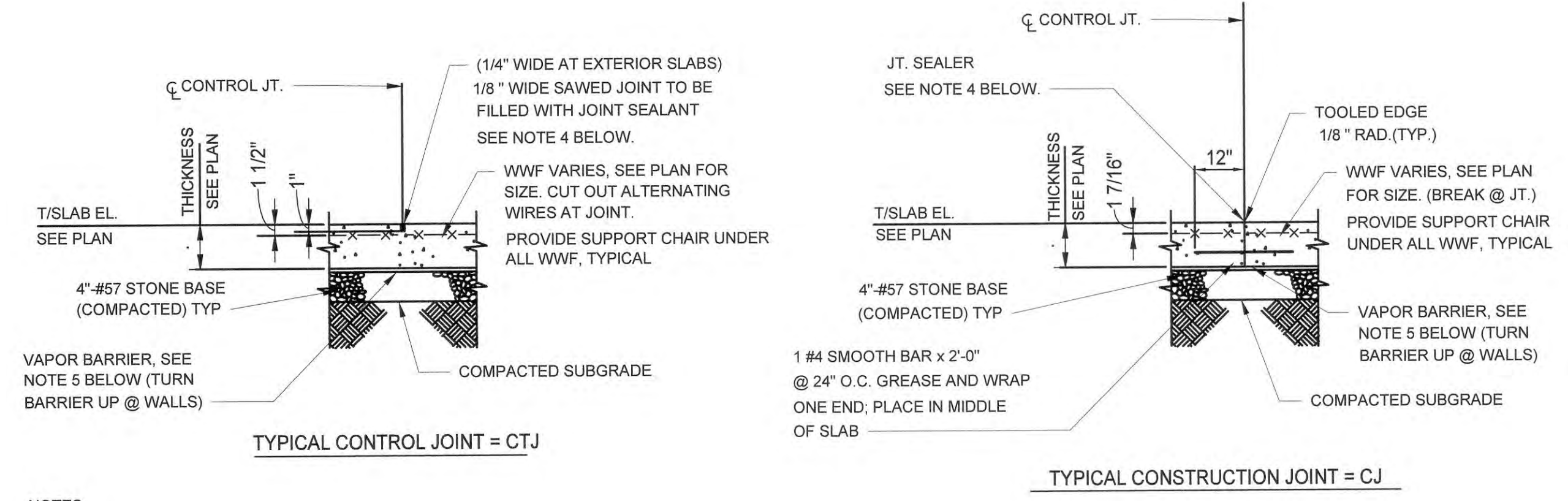
Drawing

SECTIONS AND  
 DETAILS

**S301**



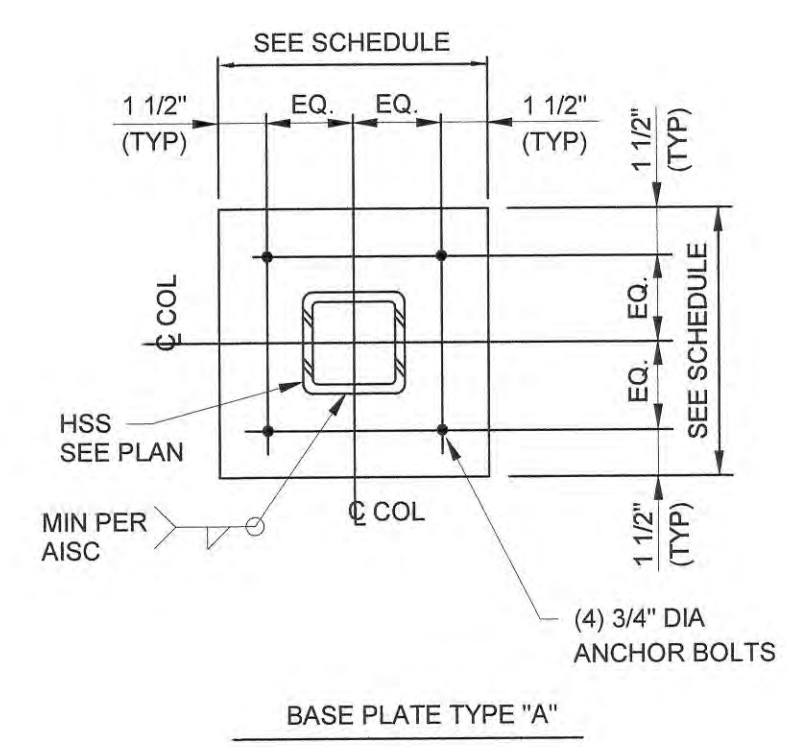
**2 SECTION**  
 S301 1" = 1'-0"



**SLAB ON GRADE JOINT DETAILS**

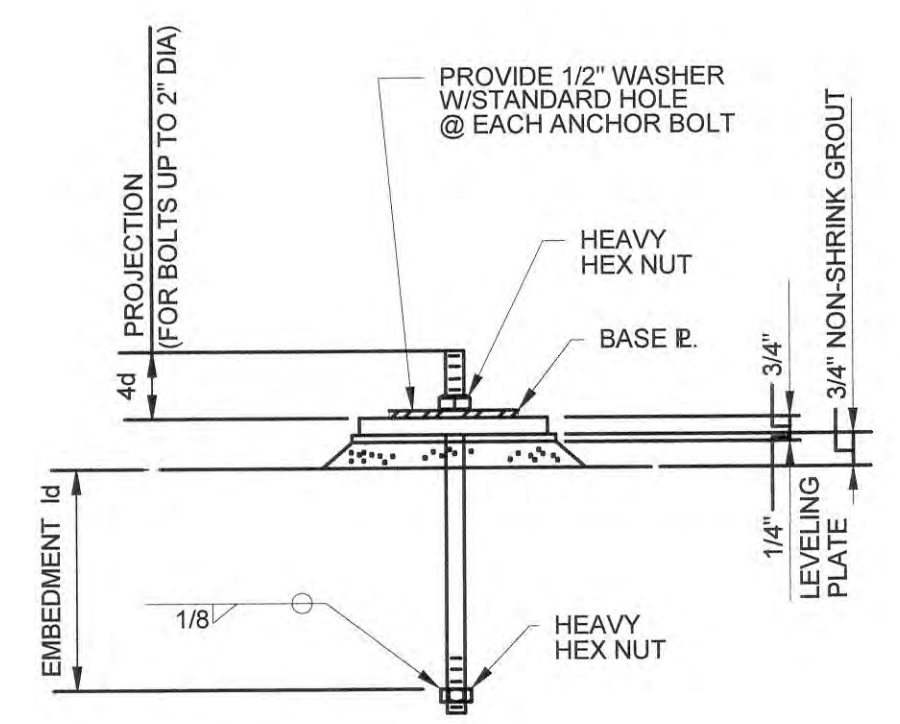
- NOTES:
- CONSTRUCTION AND/OR CONTROL JOINTS ARE INTERCHANGEABLE AT THE CONTRACTOR'S OPTION. SEE PLANS FOR LAYOUT OF CONSTRUCTION AND/OR CONTROL JOINTS. JOINTS 20' ON CENTER MAXIMUM. JOINTS LOCATED AT COLUMN CENTER LINES ARE CONSIDERED OPTIMUM.
  - DO NOT LOCATE CONSTRUCTION AND/OR CONTROL JOINTS BELOW MASONRY WALLS.
  - SAW CUTTING OF JOINTS SHALL BE AS SOON AS THE CONCRETE SETS SUFFICIENTLY TO PERMIT CUTTING WITHOUT CHIPPING SPALLING OR TEARING, BUT NOT MORE THAN 24 HOURS AFTER PLACING.
  - JOINT FILLER/SEALER SHALL BE AN ELASTOMERIC JOINT (POLYURETHANE POURABLE SEALANT) PER ASTM C920-87. (AIR TIGHT FOR RADON ELIMINATION SYSTEM).
  - VAPOR BARRIER SHALL BE 15 MIL POLYETHYLENE SHEETING OR EQUIVALENT (INSTALL PER ASTM E-1643)

**1 S.O.G JOINT DETAIL**  
 S301 3/4" = 1'-0"



BASE PLATE SCHEDULE			
COLUMN SIZE	WIDTH (IN.) B	LENGTH (IN.) N	THICKNESS (IN.) TP
HSS 10x6	15	11	3/4"
HSS 6x6	11	11	3/4"

**BASE PLATE TYPES**  
 (SEE DET. 4/S301 FOR ANCHOR BOLT DETAIL)



ld = 9" @ 3/4" DIA BOLTS  
 d = ANCHOR BOLT DIA  
 PROVIDE 1/2" WASHER WITH STANDARD HOLE @ EACH ANCHOR BOLT  
 HEAVY HEX NUT  
 BASE PL.  
 3/4"  
 1/8"  
 HEAVY HEX NUT  
 3/4" NON-SHRINK GROUT  
 LEAFING PLATE  
 1/4"  
 3/4"  
 CONTRACTOR TO VERIFY PROJECTIONS BEFORE INSTALLING ANCHOR BOLTS (SEE DETAIL 3/S301 FOR BASE PLATE DETAIL)

**4 TYP ANCHOR BOLT DETAIL**  
 S301 3/4" = 1'-0"

**3 BASE PLATE TYPES**  
 S301 3/4" = 1'-0"

REINFORCING SCHEDULE FOR COLUMN FOOTINGS					
DESIGNATION	WIDTH	LENGTH	DEPTH	LONGITUDINAL	TRANSVERSE
F3	3'-0"	3'-0"	1'-0"	3 #4 TOP 3 #4 BOTT.	3 #4 TOP 3 #4 BOTT.
F4	4'-0"	4'-0"	1'-0"	4 #4 TOP 4 #4 BOTT.	4 #4 TOP 4 #4 BOTT.
F6.5	3'-0"	6'-6"	1'-3"	4 #7 TOP 4 #7 BOTT.	7 #5 TOP 7 #5 BOTT.

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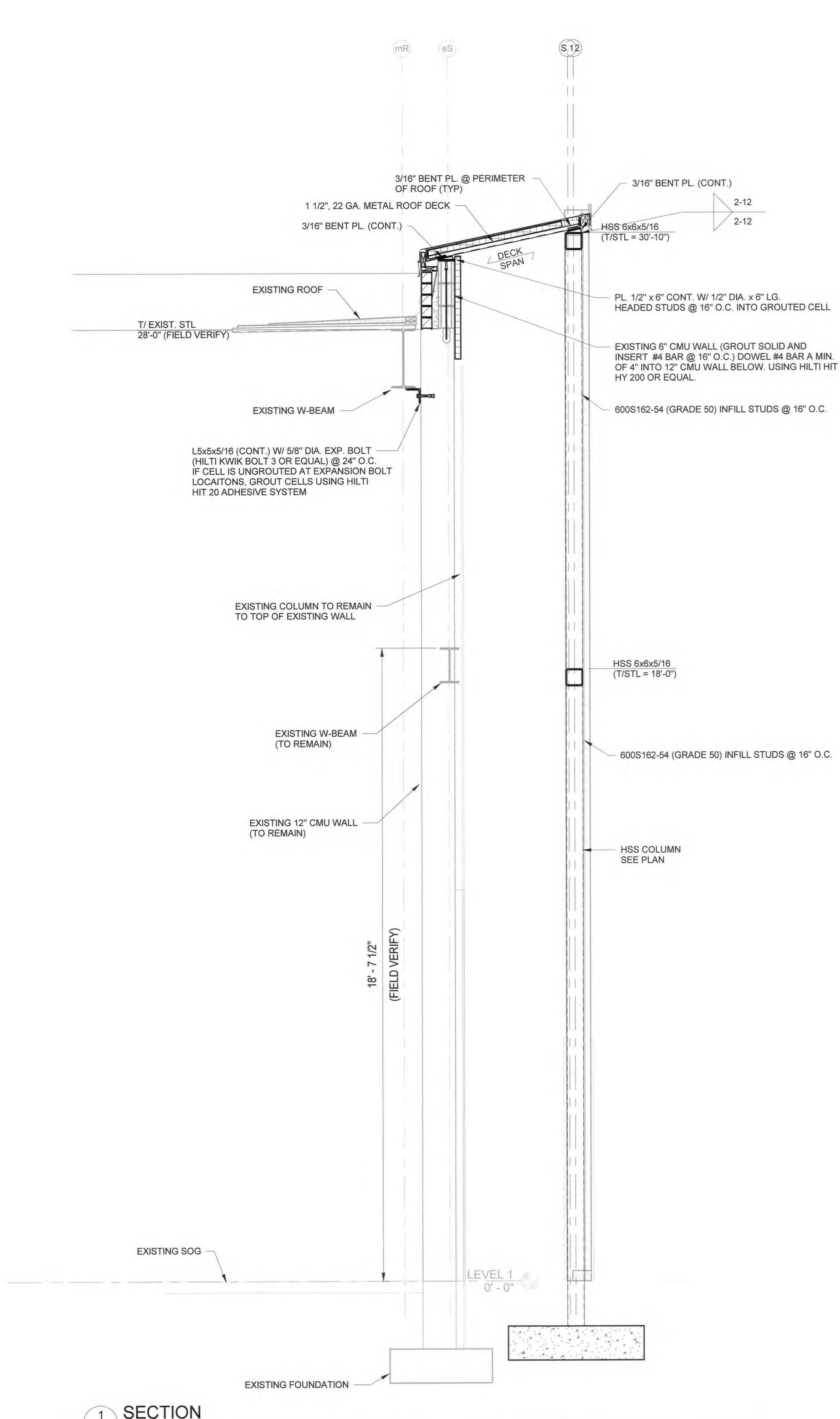
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 Project Number 15288  
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 Checked By MMS  
 Date 12 Dec 2016

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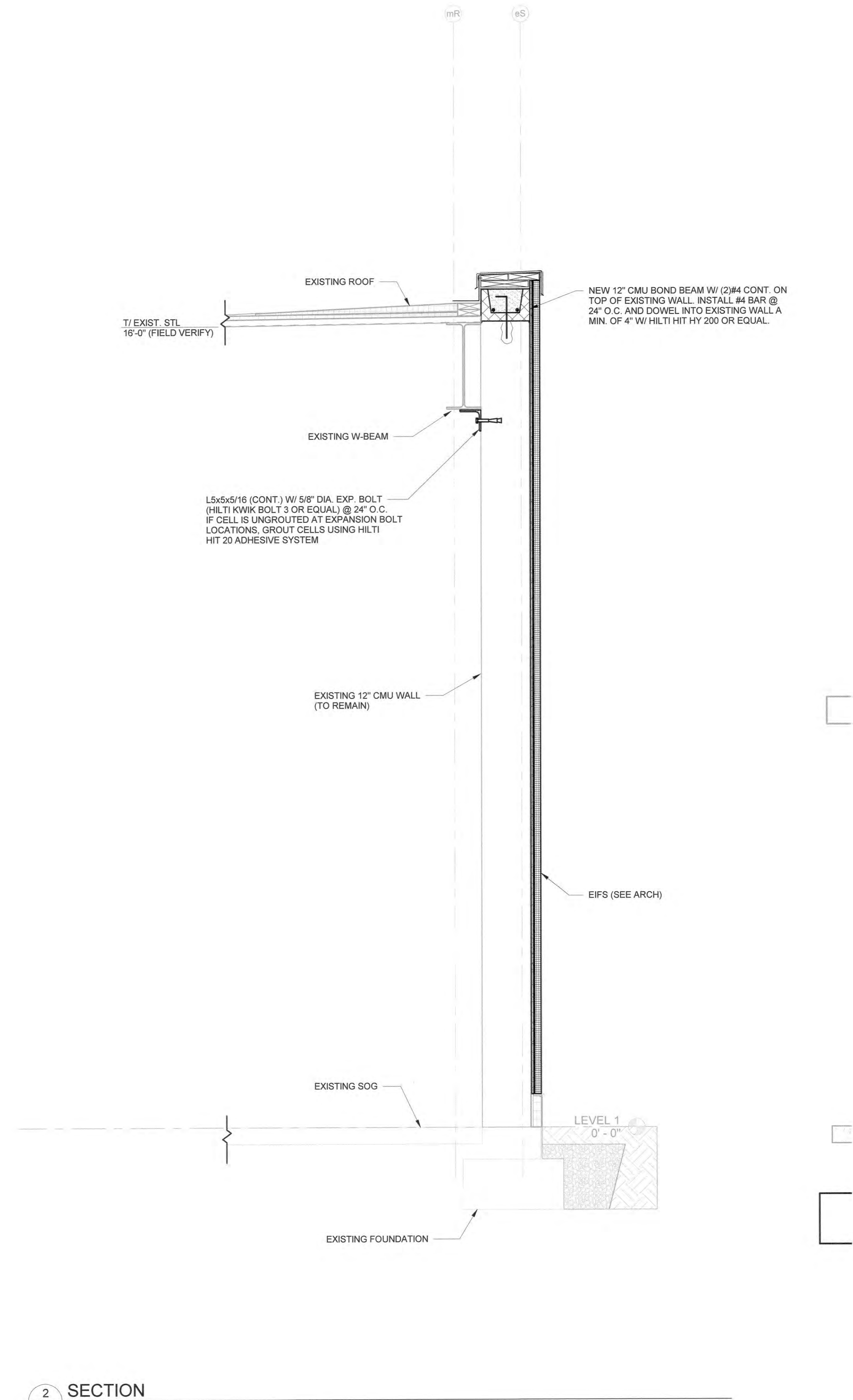
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SECTIONS AND  
 DETAILS

**S302**



1 SECTION  
 S302 1/2" = 1'-0"



2 SECTION  
 S302 3/4" = 1'-0"

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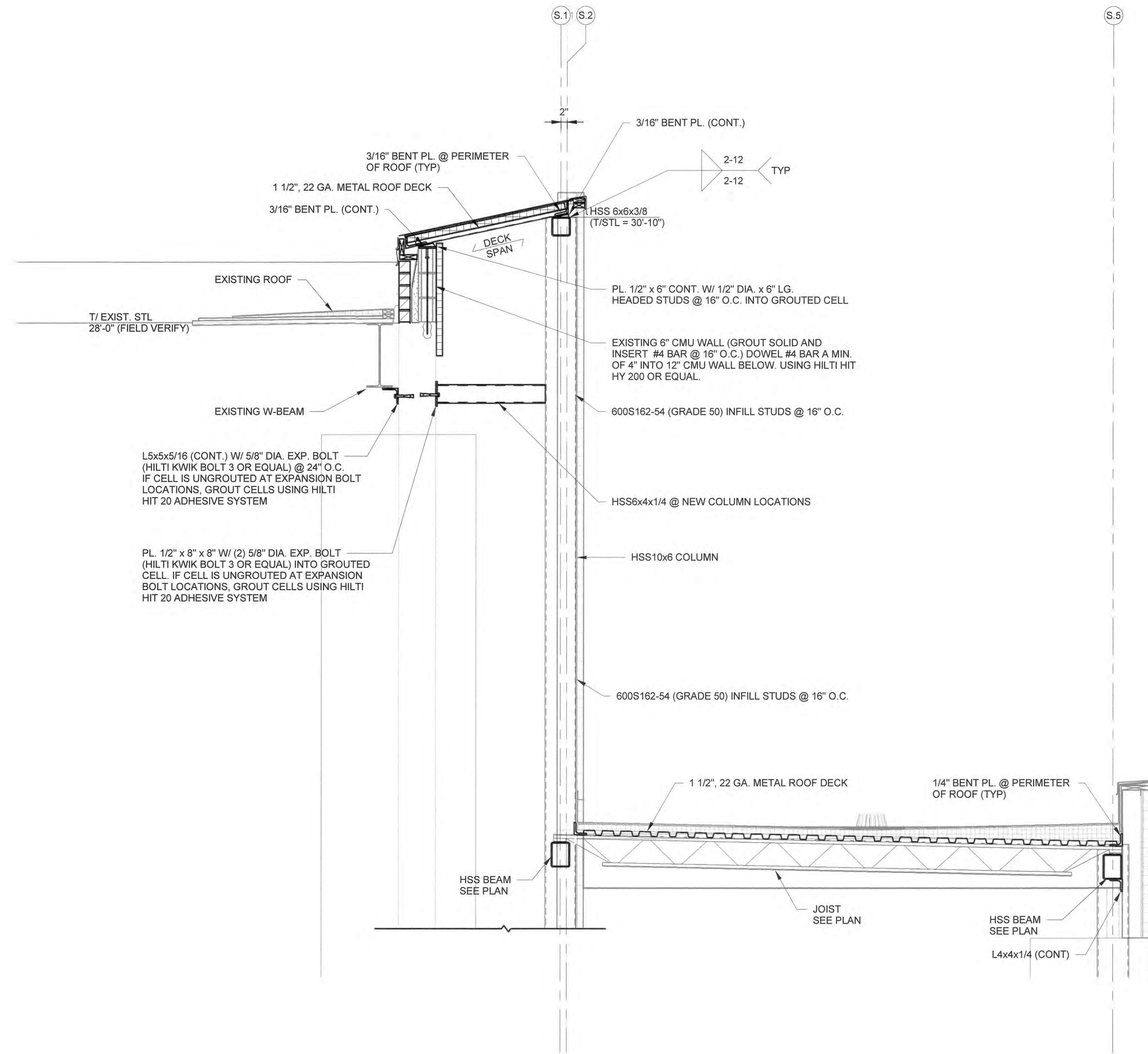
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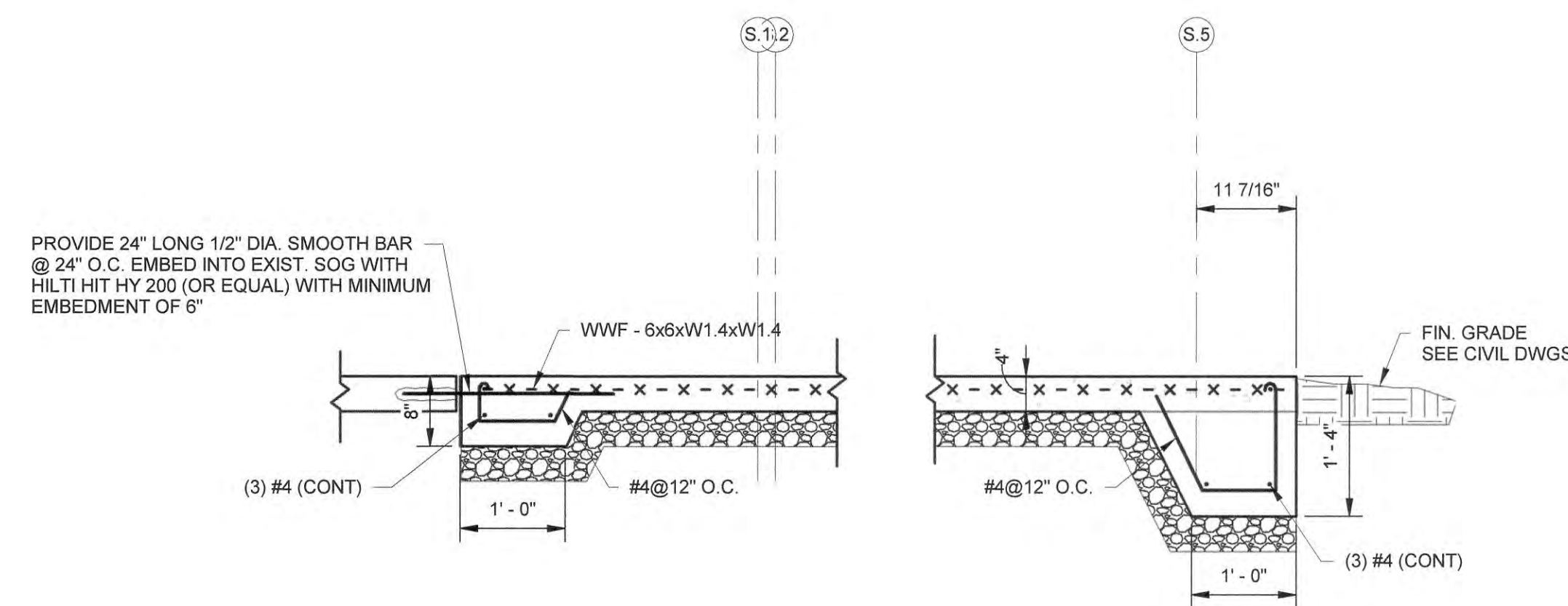
Drawing

SECTIONS AND  
 DETAILS

**S303**

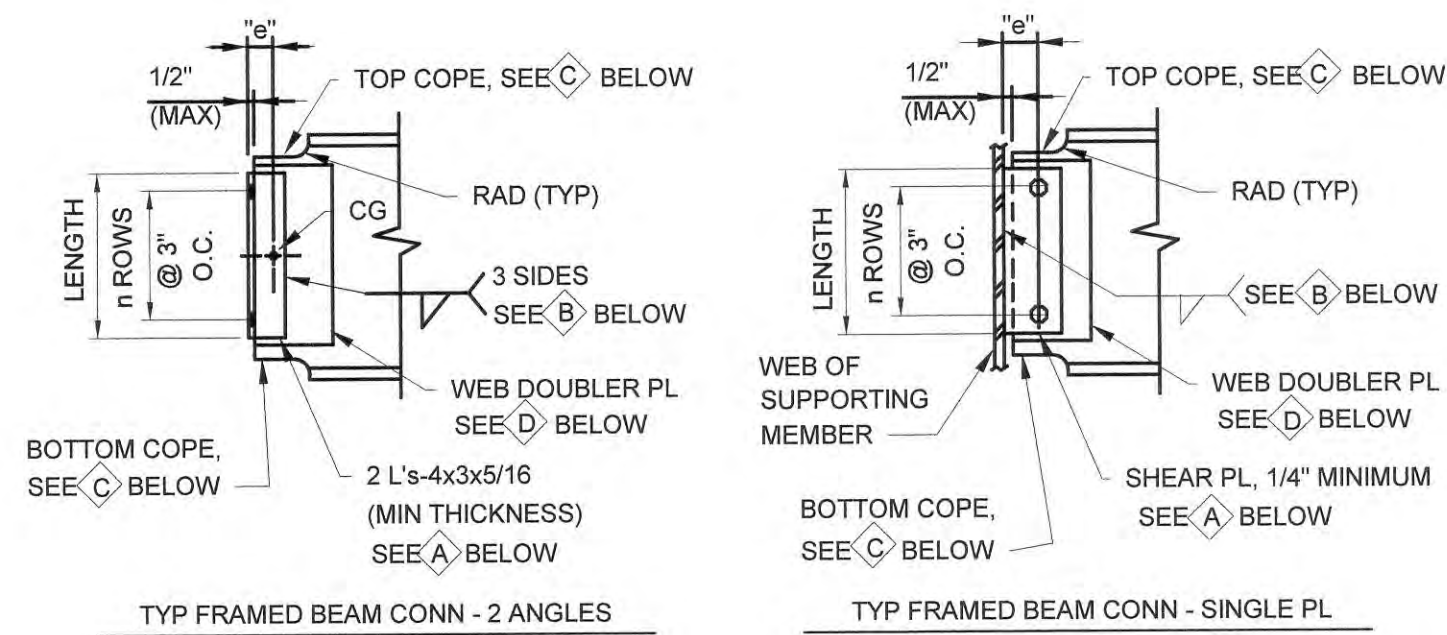


**1 SECTION**  
 S303 1/2" = 1'-0"



**2 SECTION**  
 S303 3/4" = 1'-0"

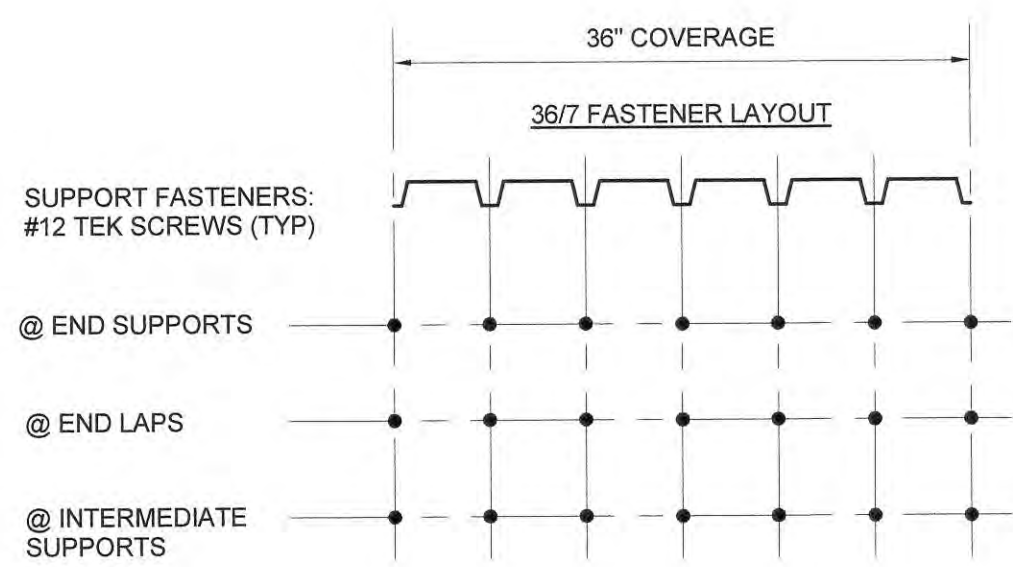
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- NOTES:
- CONNECTIONS ABOVE ARE FOR PICTORIAL REPRESENTATION ONLY AND MAY NOT BE INCLUSIVE. WEB EXTENSION PLATES, STIFFENERS, ETC SHALL BE ADDED TO FACILITATE CONNECTION LENGTH REQUIRED @ NO ADDITIONAL COST.
  - FOR BEAMS W/ AXIAL FORCES (PRying ACTION), SEE NOTE 6.0 ON SHT S001.
  - CONNECTIONS ABOVE SHALL BE DESIGNED TO DEVELOP A MIN END REACTION OF 12K.
  - CONNECTIONS SHALL BE DESIGNED SUCH THAT ALL ADDITIONAL STRESSES DUE TO ECCENTRICITY SHALL BE DEVELOPED BY THE CONNECTION AND NOT INDUCE ANY ADDITIONAL STRESSES INTO SUPPORTING MEMBERS.
  - IN ADDITION TO THE REACTION INDICATED, WEB BOLTS MUST BE DESIGNED FOR THE MOMENT IN THE BOLT GROUP CREATED AS A RESULT OF THE FORCE TRANSFER FROM THE CONNECTING MEMBER TO THE SUPPORTING MEMBER.

- A THICKNESS AND LENGTH OF ANGLES OR SHEAR PLATE AND NUMBER OF BOLTS SHALL BE DESIGNED FOR THE REACTION INDICATED USING THE CORRESPONDING "e" DISTANCE CALCULATED FOR THE CONNECTION LAYOUT AND VERIFYING ALL APPLICABLE LIMIT STATES.
- B LENGTH AND SIZE OF WELD SHALL BE DESIGNED FOR THE REACTION INDICATED INCLUDING ANY ADDITIONAL STRESS/MOMENTS CREATED IN THE LOAD TRANSFER FROM THE CONNECTING MEMBER TO THE SUPPORTING MEMBER.
- C BEAM COPES SHALL BE PROVIDED AS NECESSARY TO FACILITATE CONNECTION FIT-UP. CAPACITIES OF MEMBERS W/ COPES (TOP OR TOP & BOTTOM) SHALL BE ANALYZED TO DETERMINE THE CRITICAL LIMIT STATE DUE TO THE REDUCED SECTION INCLUDING BUT NOT LIMITED TO BUCKLING OR YIELDING OF THE WEB.
- D WEB DOUBLER PLATES OR STIFFENERS (NOT SHOWN) SHALL BE PROVIDED AS NECESSARY TO DEVELOP THE REACTION INDICATED AND FOR REGIONS OF OVER-STRESS AS DESCRIBED IN C ABOVE.

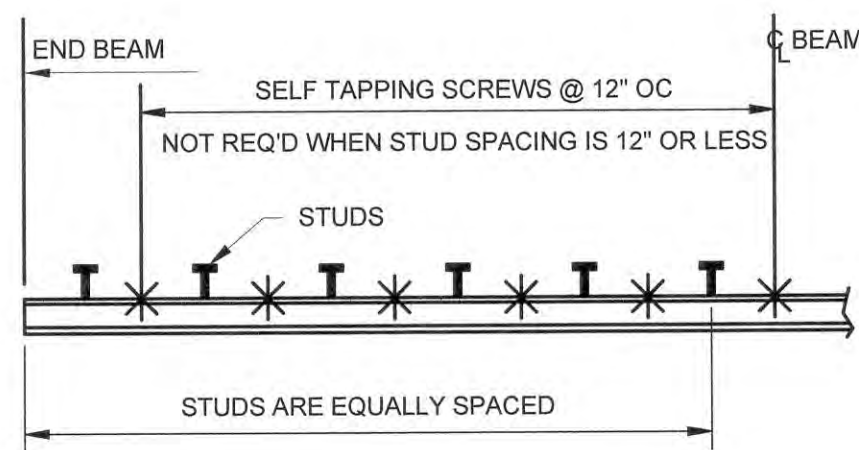
1 TYP STANDARD FRAMING DETAIL 3/32  
S401 3/4" = 1'-0"



- NOTES:
- NOTIFY ENGINEER IF ANY WIDTH OTHER THAN 36" DECK IS USED.
  - SIDELAP FASTENERS BETWEEN DECK PANELS SHALL BE (3)-#10 TEK SCREWS EQUALLY SPACED BETWEEN ADJACENT JOISTS (UNO).

METAL ROOF DECK FASTENING PATTERN

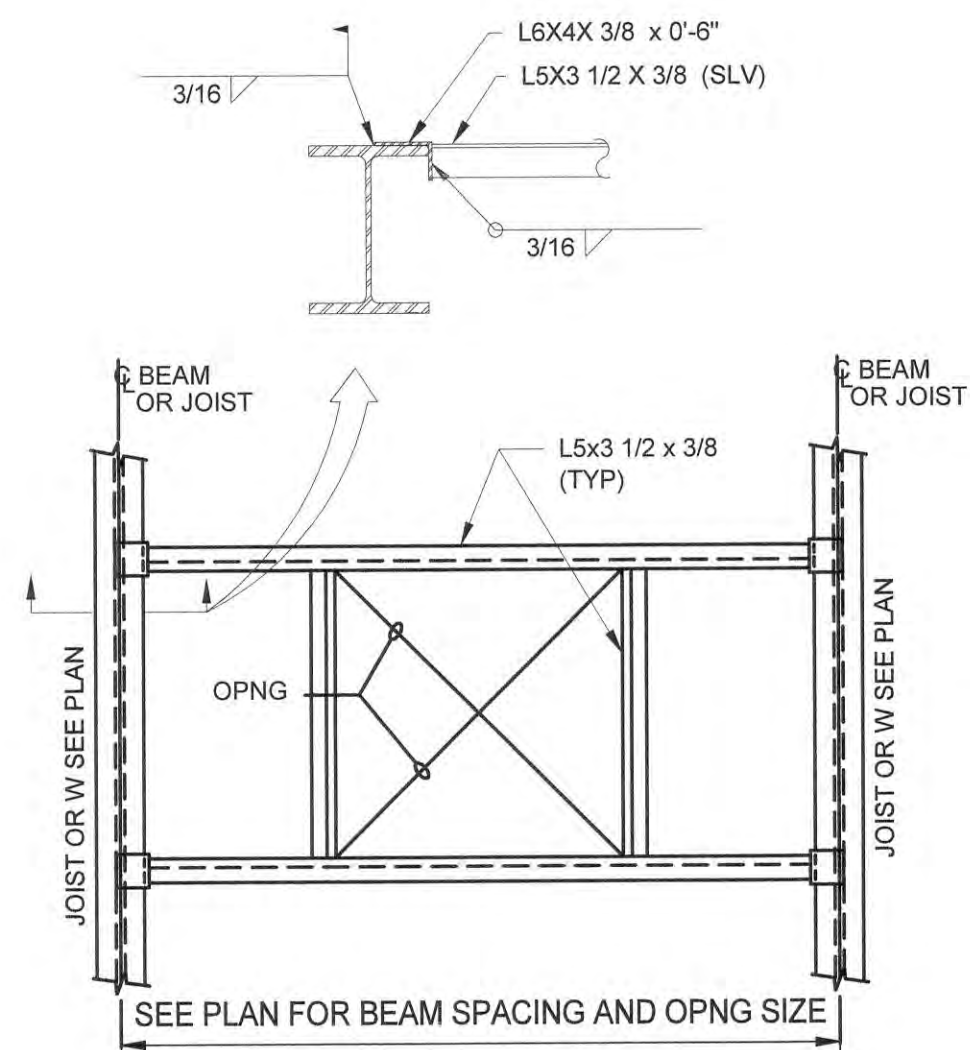
5 METAL ROOF DECK FASTENING PATTERN  
S401 3/4" = 1'-0"



3/4" DIA x 4" LG STUDS @ 4 1/2" THICK CONC SLABS  
SEE NOTE 5.5 ON DRAWING S001 FOR STUD PLACEMENT

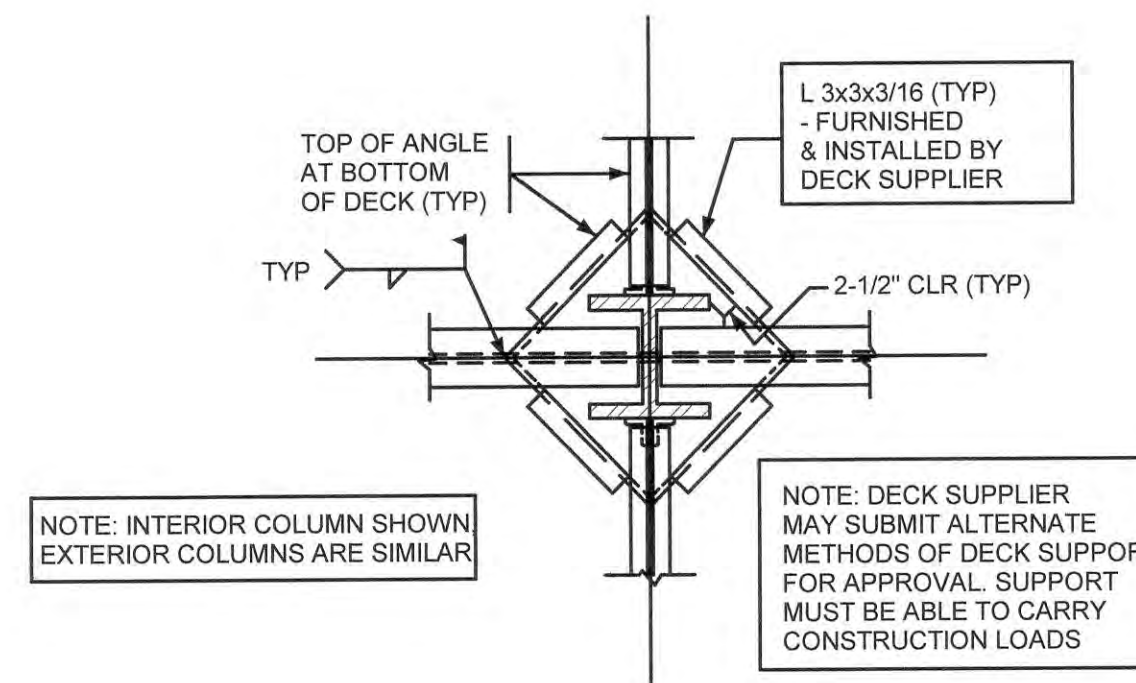
TYPICAL SHEAR CONNECTION DETAIL

2 TYP SHEAR CONNECTION DETAIL  
S401 3/4" = 1'-0"



TYPICAL PLAN FOR ROOF OPENING OR FLOOR OPENING  
PLAN OPENING 10" TO 60" OR 6" DIA TO 36" DIA

6 DETAIL AT ROOF OR FLOOR OPENING  
S401 3/4" = 1'-0"

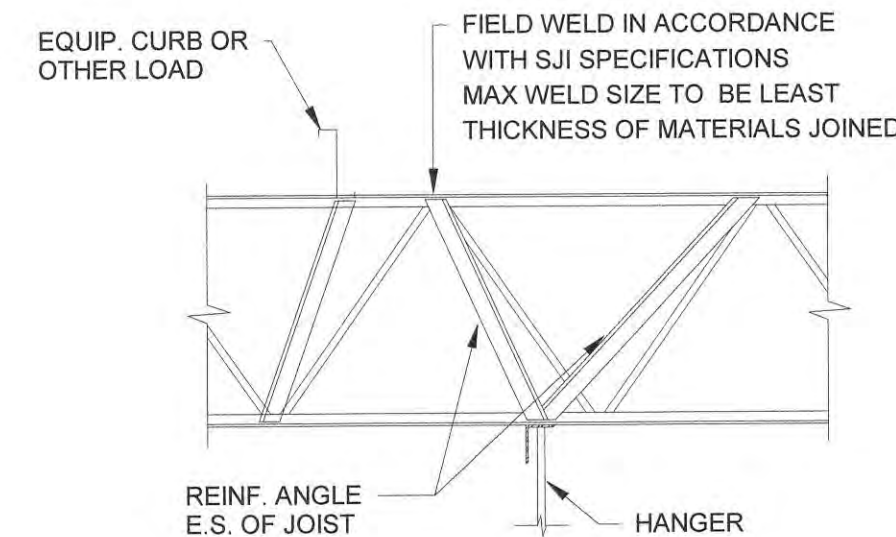


NOTE: INTERIOR COLUMN SHOWN  
EXTERIOR COLUMNS ARE SIMILAR

NOTE: DECK SUPPLIER  
MAY SUBMIT ALTERNATE  
METHODS OF DECK SUPPORT  
FOR APPROVAL. SUPPORT  
MUST BE ABLE TO CARRY  
CONSTRUCTION LOADS

TYP COMPOSITE DECK  
SUPPORT @ COL

3 TYP COMPOSITE DECK SUPPORT  
S401 3/4" = 1'-0"

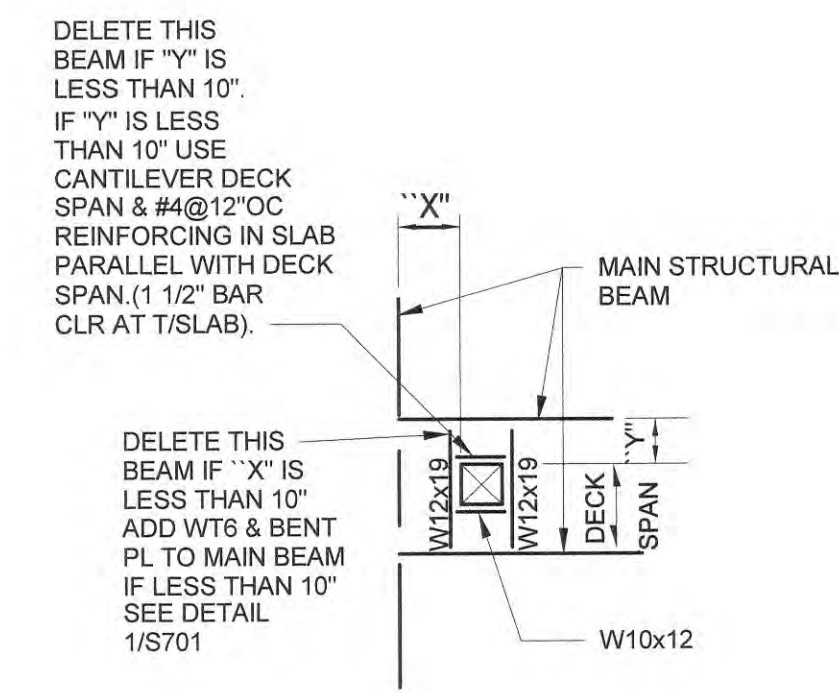


TYPICAL JOIST REINFORCEMENT  
FOR HANGER OR TOP CHORD LOADS GREATER  
THAN (A) LBS AT MORE THAN SPECIFIED MAX  
DISTANCE FROM PANEL POINTS. MAX LOAD PER  
JOIST - (B) LBS

CONCENTRATED LOADS ON JOISTS

JOIST DEPTH	JOIST REINF. ANGLE	MAX. DIST. FROM PANEL POINT TO CONCEN. LOAD W/O REINF. JOIST	A	B
UP TO 12"	L1x1x 1/8	2"	100#	300#
14" TO 18"	L1 1/4 x 1 1/4 x 1/8	3"	150#	450#
20" TO 24"	L1 1/2x1 1/2x 3/16	4"	175#	700#
26" TO 44"	L2 x 2 x 3/16	5"	200#	1000#

7 TYP. SECTION FOR CONCENTRATED LOADS ON JOISTS  
S401 3/4" = 1'-0"

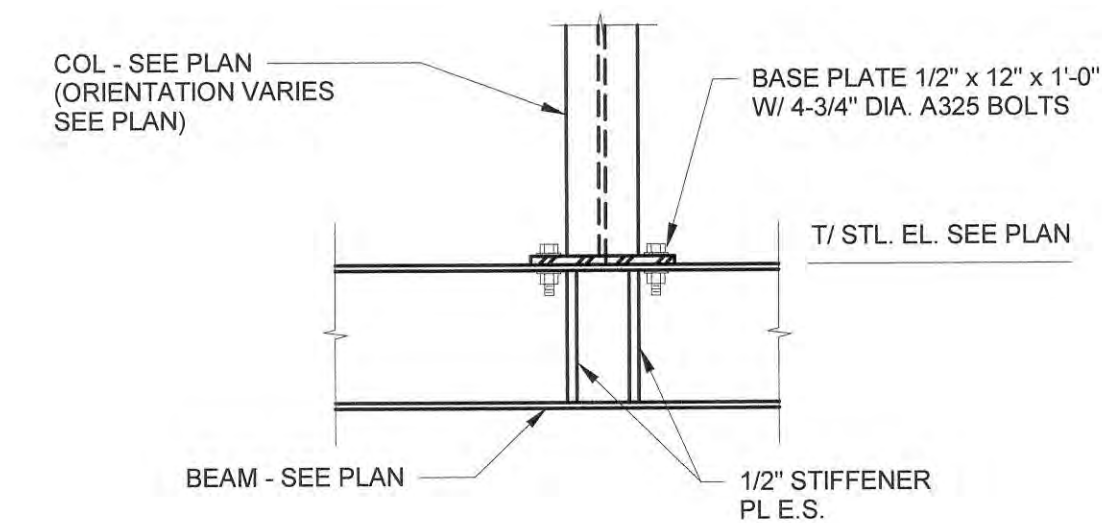


DELETE THIS BEAM IF "Y" IS LESS THAN 10".  
IF "Y" IS LESS THAN 10" USE CANTILEVER DECK SPAN & #4@12"OC REINFORCING IN SLAB PARALLEL WITH DECK SPAN (1 1/2" BAR CLR AT T/SLAB).

DELETE THIS BEAM IF "X" IS LESS THAN 10".  
ADD WTB & BENT PL TO MAIN BEAM IF LESS THAN 10" SEE DETAIL 1/5701

TYPICAL MECHANICAL PENETRATION  
THROUGH COMPOSITE DECK

4 TYP. FRAMING OPENING IN COMPOSITE SLAB  
S401 3/4" = 1'-0"



8 POST CONNECTION DETAIL  
S401 3/4" = 1'-0"

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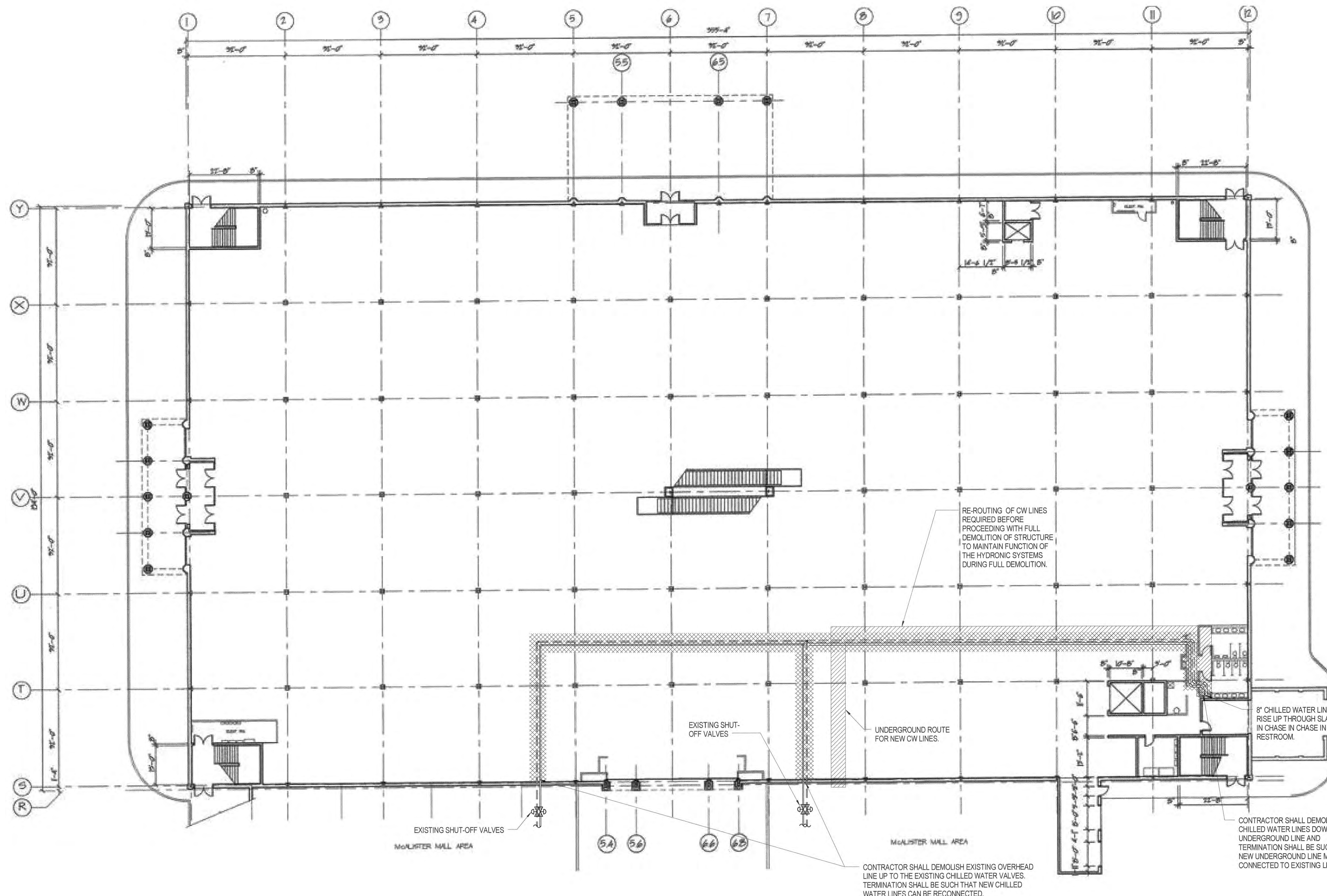
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Revisions

Drawing

SECTIONS AND  
DETAILS

S401



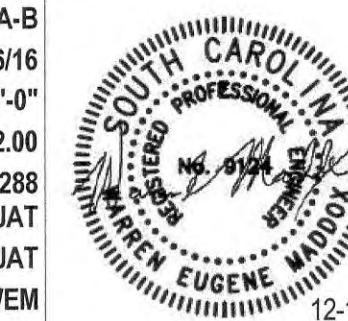
KEY PLAN:



#	Revision	Date

**DEMOLITION OF BUILDING 602**

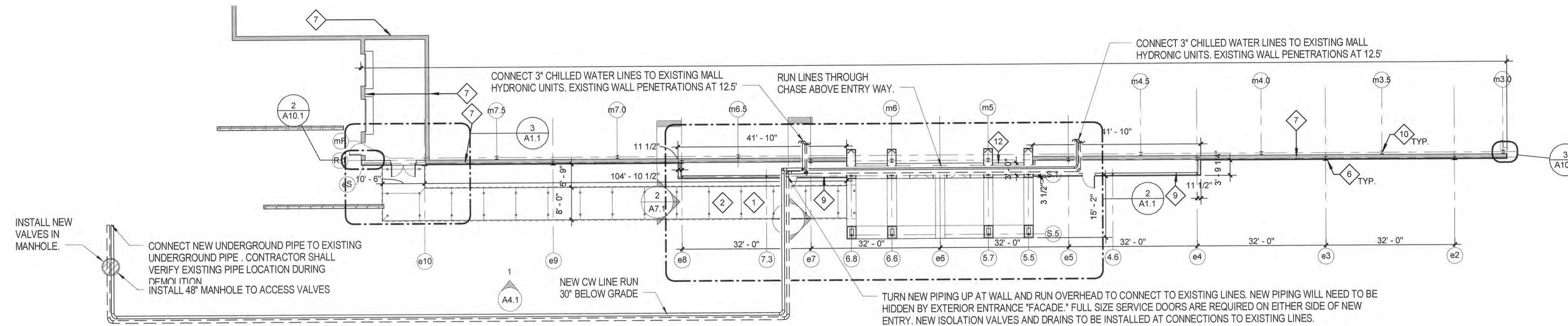
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DATE: 10/26/16  
SCALE: 1/16" = 1'-0"  
EYP PROJECT NO.: 1015032.00  
CLIENT PROJECT NO.: 15288  
DESIGNED BY: JAT  
DRAWN BY: JAT  
CHECKED BY: WEM



**MECHANICAL DEMOLITION PLAN**

**M1.1**

**1 BELK DEMOLITION PLAN**  
M1.1 SCALE: 1/16" = 1'-0"



**1 BELK NEW CONSTRUCTION PLAN**  
SCALE: 1/16" = 1'-0"

**SCOPE OF WORK:**

1. DEMOLISH WALLS AND FIXTURES SURROUNDING THE MAIN 8" CW LINES IN THE RESTROOM.
2. CONTRACTOR SHALL TRENCH BELOW GRADE AT A MINIMUM 30" FOR NEW CW LINES. APPROXIMATE TO PATH SHOWN ON DEMOLITION PLAN.
3. ROUTE NEW LINES IN TRENCH AND UP THROUGH CHASE IN THE FACADE AT MALL ENTRANCE. ADD ISOLATION VALVES TO NEW CW LINES AT BOTH 3" LINES AND AT THE 8" MAIN.
4. FLUSH DEBRIS IN NEW CW LINES.
5. COORDINATE SHUTDOWN OF HVAC SYSTEMS WITH GREENVILLE TECH.
6. CLOSE ALL EXISTING VALVES AND DISCONNECT OLD CW LINES.
7. TIE-IN NEW OVERHEAD CW LINES (SEE DETAIL 3 & 4 ON SHEET M1.3 FOR HANGERS AND SUPPORTS).
8. TIE-IN NEW UNDERGROUND CW LINES (SEE DETAIL 2 ON SHEET M1.3).
9. BACKFILL TRENCH.
10. TURN ON HVAC SYSTEMS TO RESTORE FULL FUNCTION OF THE HYDRONIC SYSTEMS.
11. CONTINUE DEMO OF STRUCTURE.

**PART 1 - PRODUCTS**

- 1.1 PLASTIC PIPE AND FITTINGS
  - A. POLYPROPYLENE OR POLYETHYLENE PIPE AND FITTINGS FOR WATER DISTRIBUTION AND WATER SERVICE
    1. PIPE SHALL BE MANUFACTURED MEETING THE SHORT-TERM PROPERTIES AND LONG-TERM STRENGTH REQUIREMENTS OF ASTM F 2389. THE PIPE SHALL CONTAIN NO REWORK OR RECYCLED MATERIALS EXCEPT THAT GENERATED IN THE MANUFACTURER'S OWN PLANT FROM RESIN OF THE SAME SPECIFICATION FROM THE SAME RAW MATERIAL. ALL PIPE SHALL BE MADE IN AN EXTRUSION PROCESS. ALL PIPE SHALL COMPLY WITH THE RATED PRESSURE REQUIREMENTS OF ASTM F 2389. ALL PIPE SHALL BE CERTIFIED BY NSF INTERNATIONAL AS COMPLYING WITH NSF 14, NSF 61, AND ASTM F 2389 OR CSA B137.11.
    2. FITTINGS SHALL BE MANUFACTURED MEETING THE SHORT-TERM PROPERTIES AND LONG-TERM STRENGTH REQUIREMENTS OF ASTM F 2389. THE FITTINGS SHALL CONTAIN NO REWORK OR RECYCLED MATERIALS EXCEPT THAT GENERATED IN THE MANUFACTURER'S OWN PLANT FROM RESIN OF THE SAME SPECIFICATION FROM THE SAME RAW MATERIAL. ALL FITTINGS SHALL BE CERTIFIED BY NSF INTERNATIONAL AS COMPLYING WITH NSF 14, NSF 61, AND ASTM F 2389 OR CSA B137.11.
    3. FITTINGS: SOCKET FUSION, BUTT FUSION, ELECTROFUSION, OR FUSION OUTLET FITTINGS SHALL BE USED FOR FUSION WELD JOINTS BETWEEN PIPE AND FITTINGS.
    4. MECHANICAL FITTINGS AND TRANSITION FITTINGS SHALL BE USED WHERE TRANSITIONS ARE MADE TO OTHER PIPING MATERIALS OR TO VALVES AND APPURTENANCES.
    5. PIPE SHALL NOT BE THREADED. THREADED TRANSITION FITTINGS PER ASTM F 2389 SHALL BE USED WHERE A THREADED CONNECTION IS REQUIRED.
    6. PIPE USED FOR HOT WATER DISTRIBUTION SHALL INCLUDE A FIBERGLASS-REINFORCED LAYER TO REDUCE THERMAL EXPANSION/CONTRACTION.
  - B. PLASTIC, PIPE-FLANGE GASKET, BOLTS, AND NUTS: TYPE AND MATERIAL RECOMMENDED BY PIPING SYSTEM MANUFACTURER UNLESS OTHERWISE INDICATED.
- 1.2 TRANSITION FITTINGS
  - A. PLASTIC-TO-METAL TRANSITION FITTINGS SHALL BE THE FOLLOWING:
    1. ONE-PIECE FITTING WITH THREADED STAINLESS STEEL, BRASS, OR COPPER INSERT AND ONE FUSION WELD JOINT END.
- 1.3 INSULATIONS
  - A. PRE-INSULATED PIPE
    1. PRE-INSULATED PIPE SHALL BE A COMPLETE SYSTEM OF FACTORY PRE-INSULATED PIPING FOR THE SPECIFIED SERVICE.
    2. INSULATION SHALL BE POLYURETHANE FOAM EITHER SPRAY APPLIED OR INJECTED WITH ONE SHOT INTO THE ANNULAR SPACE BETWEEN CARRIER PIPE AND JACKET WITH A THICKNESS OF TWO INCHES. INSULATION SHALL BE RIGID, 90-95% CLOSED CELL POLYURETHANE WITH A 2.0 TO 3.0 POUNDS PER CUBIC FOOT DENSITY AND COEFFICIENT OF THERMAL CONDUCTIVITY (K-FACTOR) OF 0.16 AND SHALL CONFORM TO ASTM C-591.
    3. JACKETING MATERIAL SHALL BE EXTRUDED, BLACK, HIGH DENSITY POLYETHYLENE (HDPE), HAVING A MINIMUM WALL THICKNESS OF 100 MILS FOR JACKET SIZES LESS THAN OR EQUAL TO 12", AND 125 MILS FOR JACKET SIZES LARGER THAN 12" TO 24".
    4. MANUFACTURERS: SUBJECT TO COMPLIANCE WITH REQUIREMENTS, AVAILABLE MANUFACTURERS OFFERING PRODUCTS THAT MAY BE INCORPORATED INTO THE WORK INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
      - a. AQUATHERM TI PIPE
      - b. INSUL-TEK PIPING SYSTEMS, INC.
      - c. PERMA-PIPE, INC.
      - d. ROVANCO PIPING SYSTEMS, INC.
      - e. THERMACOR PROCESS, L.P.
      - f. TRICON PIPING SYSTEMS, INC.

**PART 2 - EXECUTION**

- 2.1 PIPING INSTALLATION
  - A. DRAWING PLANS, SCHEMATICS, AND DIAGRAMS INDICATE GENERAL LOCATION AND ARRANGEMENT OF PIPING SYSTEMS. INDICATE PIPING LOCATIONS AND ARRANGEMENTS IF SUCH WERE USED TO SIZE PIPE AND CALCULATE FRICTION LOSS, EXPANSION, PUMP SIZING, AND OTHER DESIGN CONSIDERATIONS. INSTALL PIPING AS INDICATED UNLESS DEVIATIONS TO LAYOUT ARE APPROVED ON COORDINATION DRAWINGS.
  - B. INSTALLERS SHALL BE TRAINED AND CERTIFIED TO INSTALL THE PIPE ACCORDING TO THE MANUFACTURER'S GUIDELINES. CONTACT YOUR LOCAL REPRESENTATIVE FOR TRAINING.
  - C. REMOVE STANDING WATER IN THE BOTTOM OF TRENCH.
  - D. DO NOT BACKFILL PIPING TRENCH UNTIL FIELD QUALITY-CONTROL TESTING HAS BEEN COMPLETED AND RESULTS APPROVED.
  - E. INSTALL PIPING AT UNIFORM GRADE OF 0.2 PERCENT. INSTALL DRAINS, CONSISTING OF A TEE FITTING, NPS 3/4 (DN 20) BALL VALVE, AND SHORT NPS 3/4 (DN 20) THREADED NIPPLE WITH CAP, AT LOW POINTS (IN MANHOLE) AND ELSEWHERE AS REQUIRED FOR SYSTEM DRAINAGE. INSTALL MANUAL AIR VENTS AT HIGH POINTS (MALL).
  - F. IN CONDUITS, INSTALL DRAIN VALVES AT LOW POINTS AND MANUAL AIR VENTS AT HIGH POINTS.
  - G. INSTALL COMPONENTS WITH PRESSURE RATING EQUAL TO OR GREATER THAN SYSTEM OPERATING PRESSURE.
  - H. INSTALL PIPING FREE OF SAGS AND BENDS.
  - I. INSTALL FITTINGS FOR CHANGES IN DIRECTION AND BRANCH CONNECTIONS.
  - J. INSTALL SLEEVES AND MECHANICAL SLEEVE SEALS THROUGH EXTERIOR BUILDING WALLS AND MANHOLE.
  - K. THRUST BLOCKS SHALL NOT BE REQUIRED.
  - L. EXPANSION LOOPS SHALL NOT BE REQUIRED FOR UNDERGROUND.
- 2.2 JOINT CONSTRUCTION
  - A. REAM ENDS OF PIPES AND TUBES AND REMOVE BURRS.
  - B. REMOVE SCALE, SLAG, DIRT, AND DEBRIS FROM INSIDE AND OUTSIDE OF PIPE AND FITTINGS BEFORE ASSEMBLY.
  - C. FUSION JOINTS: FUSION JOIN PIPE IN ACCORDANCE WITH ASTM D2857, ASTM F 2389, AND THE MANUFACTURER'S INSTRUCTIONS.
  - D. THREADED JOINTS: THREAD PIPE WITH TAPERED PIPE THREADS ACCORDING TO ASME B1.20.1. JOIN PIPE FITTINGS AND VALVES AS FOLLOWS:
    1. APPLY APPROPRIATE TAPE OR THREAD COMPOUND TO EXTERNAL PIPE THREADS UNLESS DRY SEAL THREADING IS SPECIFIED.
    2. DAMAGED THREADS: DO NOT USE PIPE OR PIPE FITTINGS WITH THREADS THAT ARE CORRODED OR DAMAGED. DO NOT USE PIPE SECTIONS THAT HAVE CRACKED OR OPEN WELDS.
  - F. FLANGED JOINTS: SELECT APPROPRIATE GASKET MATERIAL, SIZE, TYPE, AND THICKNESS FOR SERVICE APPLICATION. INSTALL GASKET CONCENTRICALLY POSITIONED. USE SUITABLE LUBRICANTS ON BOLT THREADS.
  - G. INSULATION JOINTS MADE FOR PRE-INSULATED PIPE SHALL BE DONE IN ACCORDANCE WITH THE INSULATION MANUFACTURER'S INSTRUCTIONS.
- 2.3 IDENTIFICATION
  - A. INSTALL CONTINUOUS METALLIC/PLASTIC UNDERGROUND WARNING TAPES DURING BACK FILLING OF TRENCHES FOR UNDERGROUND HYDRONIC PIPING. LOCATE TAPES 6 TO 8 INCHES TO 200 MM) BELOW FINISHED GRADE, DIRECTLY OVER PIPING ALTERNATIVELY INSTALL 8 - 10 GAGE COPPER WIRE AT 6" - 8" DIRECTLY OVER THE PIPELINE. PROVIDE WARNING TAPES ABOVE THE WIRE AT 6" TO 8" BELOW THE FINISHED GRADE DIRECTLY OVER THE PIPELINE.
- 2.4 FIELD QUALITY CONTROL
  - A. WHILE STILL ACCESSIBLE ALL PIPING SHALL BE PRESSURE/LEAK TESTED TO THE MANUFACTURER'S STANDARDS. TESTS SHALL BE CARRIED OUT USING WATER, COMPRESSED AIR OR A MIXTURE OF THE TWO. THE TEST PRESSURE SHALL BE AS INDICATED IN THE PRESSURE LEAK TESTING PROCEDURES REQUIRED BY THE MANUFACTURER. ANY LEAKS DETECTED SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE BY REMOVING THE LEAKING PART AND REPLACING WITH NEW PARTS WELDED PER THE PIPE MANUFACTURER'S GUIDELINES. SEE WWW.AQUATHERM.COM FOR ADDITIONAL DETAILS AND FORMS.
  - B. PREPARE TEST AND INSPECTION REPORTS. DELIVER TO OWNER AND MANUFACTURER TO OBTAIN PRODUCT WARRANTY.

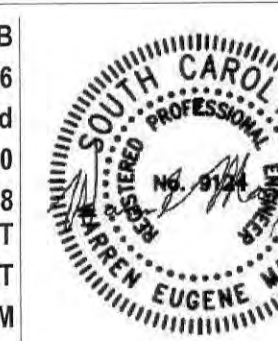
KEY PLAN:



#	Revision	Date

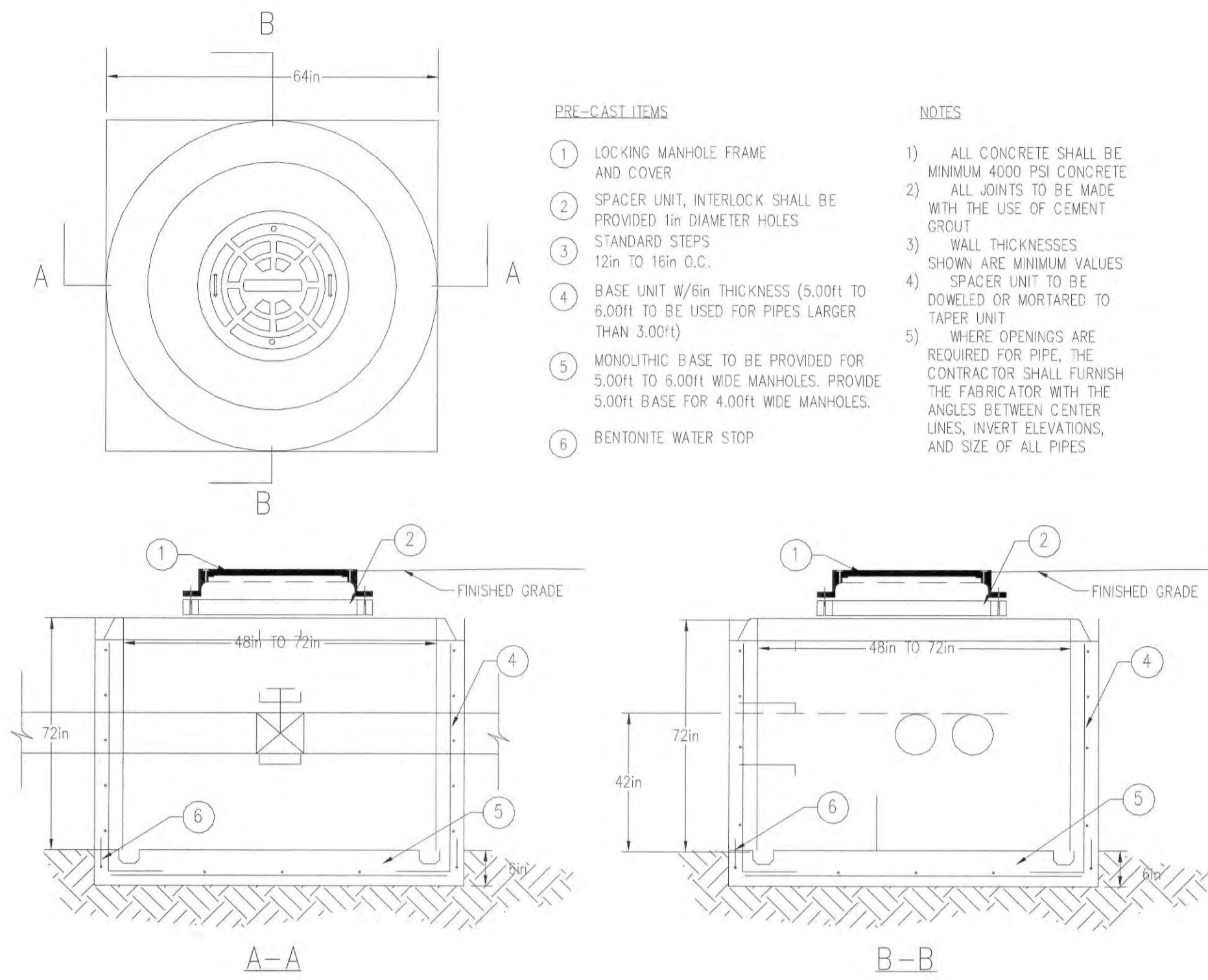
**DEMOLITION OF BUILDING 602**

STATE PROJECT NO.: H59-6124-CA-B  
DATE: 10/26/16  
SCALE: As indicated  
EYP PROJECT NO.: 1015032.00  
CLIENT PROJECT NO.: 15288  
DESIGNED BY: JAT  
DRAWN BY: JAT  
CHECKED BY: WEM



**MECHANICAL NEW CONSTRUCTION PLAN**

**M1.2**



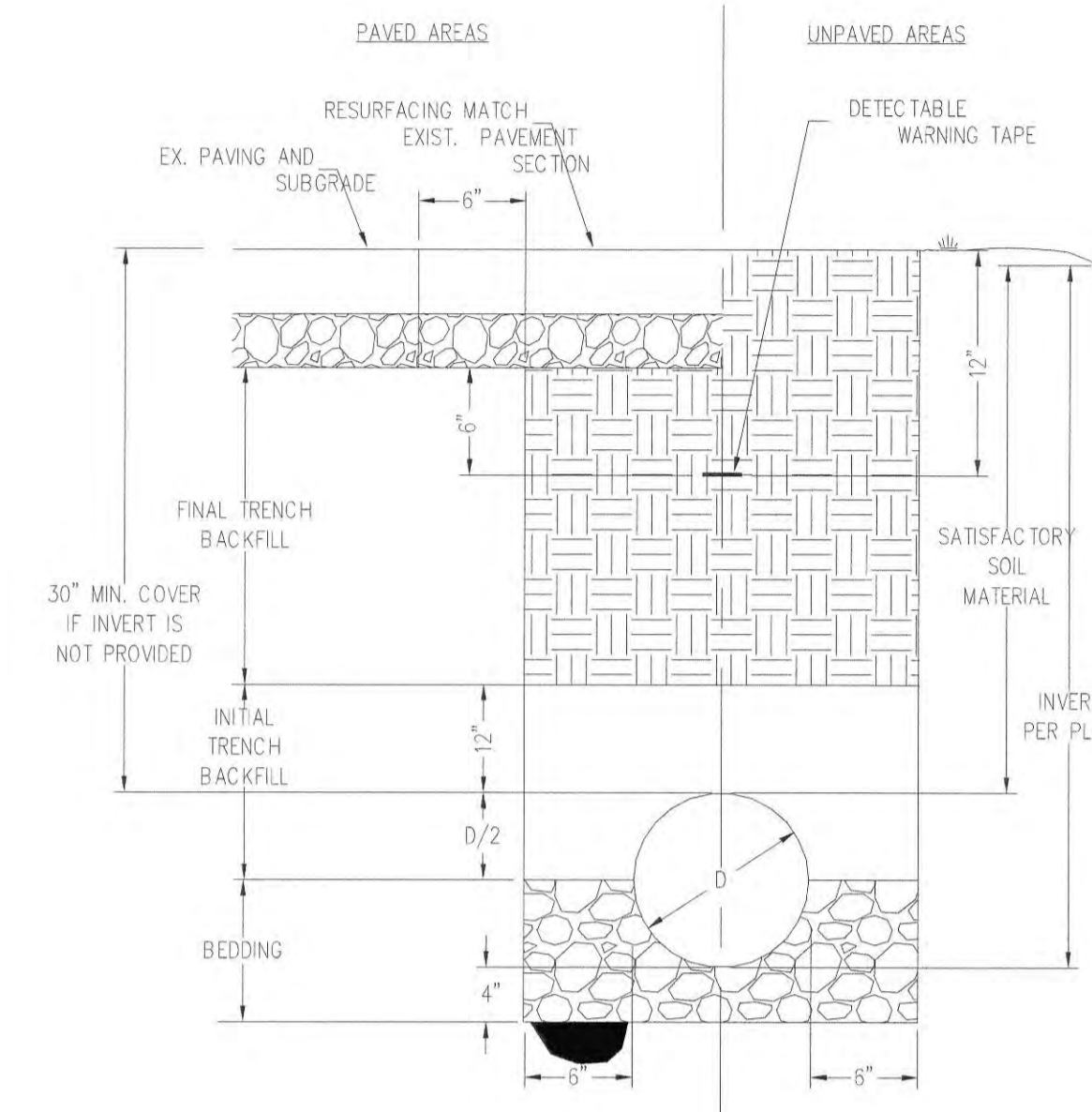
PRE-CAST ITEMS

- 1) LOCKING MANHOLE FRAME AND COVER
- 2) SPACER UNIT, INTERLOCK SHALL BE PROVIDED 1in DIAMETER HOLES
- 3) STANDARD STEPS 12in TO 16in D.C.
- 4) BASE UNIT W/6in THICKNESS (5.00ft TO 6.00ft TO BE USED FOR PIPES LARGER THAN 3.00ft)
- 5) MONOLITHIC BASE TO BE PROVIDED FOR 5.00ft TO 6.00ft WIDE MANHOLES. PROVIDE 5.00ft BASE FOR 4.00ft WIDE MANHOLES.
- 6) BENTONITE WATER STOP

NOTES

- 1) ALL CONCRETE SHALL BE MINIMUM 4000 PSI CONCRETE
- 2) ALL JOINTS TO BE MADE WITH THE USE OF CEMENT GROUT
- 3) WALL THICKNESSES SHOWN ARE MINIMUM VALUES
- 4) SPACER UNIT TO BE DOWELED OR MORTARED TO TAPER UNIT
- 5) WHERE OPENINGS ARE REQUIRED FOR PIPE, THE CONTRACTOR SHALL FURNISH THE FABRICATOR WITH THE ANGLES BETWEEN CENTER LINES, INVERT ELEVATIONS, AND SIZE OF ALL PIPES

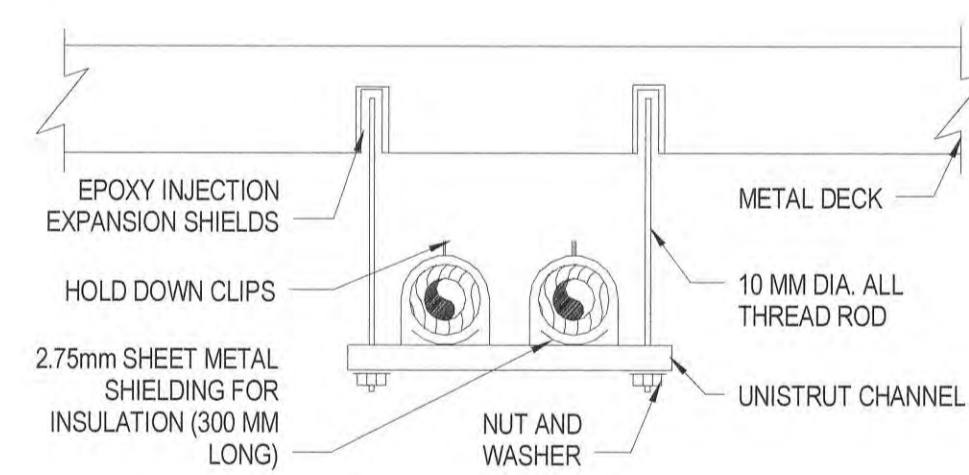
1 MANHOLE  
SCALE: NTS



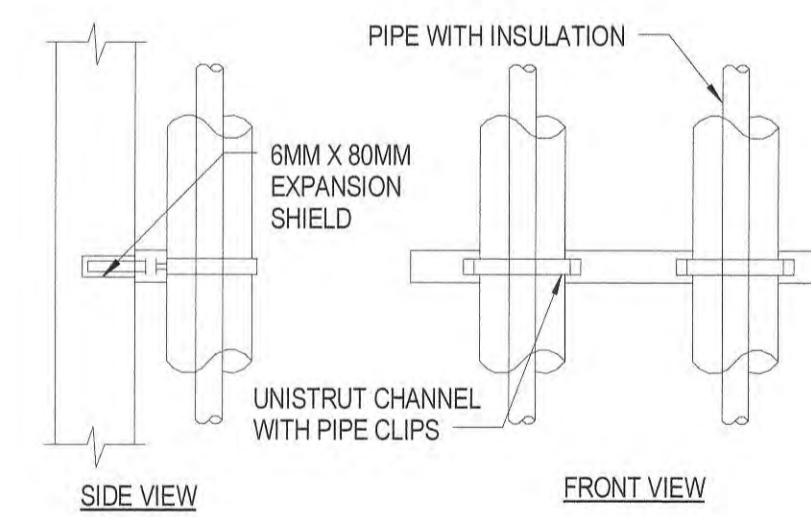
NOTES:

1. PIPE BEDDING AND BACKFILL SHALL BE OF SUITABLE ONSITE MATERIAL (FREE OF GRAVEL LARGER THAN 1"), #57 STONE, OR SAND, COMPACT TO A MINIMUM OF 90% STANDARD PROCTOR DENSITY. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.

2 PIPE BEDDING AND BACKFILL  
SCALE: NTS



3 TRAPEZE PIPE HANGER  
SCALE: NTS



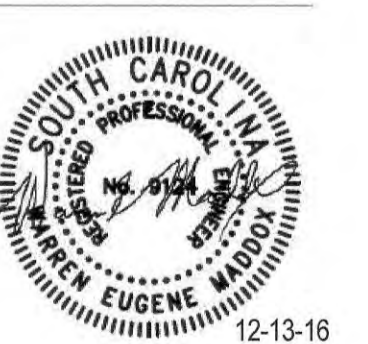
4 WALL SUPPORT FOR VERTICAL PIPING  
SCALE: NTS

KEY PLAN:

#	Revision	Date

DEMOLITION OF BUILDING 602

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12-13-16